# GATE 2023 

## Civil Engineering

## Forenoon Shift

## Questions with Detailed Solutions

## General Aptitude

1. "I have not yet decided what I will do this evening; I $\qquad$ visit a friend."
A. Mite
B. Would
C. Might
D. Didn't
[MCQ - 1 Mark]
Ans. C
Sol. "I might visit a friend"
2. Eject: Insert :: Advance : $\qquad$
(By word meaning)
A. Advent
B. Progress
C. Retreat
D. Loan
[MCQ - 1 Mark]
Ans. C
Sol. Eject: Insert :: Advance: Retreat
3. In the given figure, PQRSTV is a regular hexagon with each side of length 5 cm . A circle is drawn with its centre at $V$ such that it passes through $P$. What is the area (in $\mathrm{cm}^{2}$ ) of the shaded region? (The diagram is representative)

A. $\frac{25 \pi}{3}$
B. $\frac{20 \pi}{3}$
C. $6 \pi$
D. $7 \pi$
[MCQ - 1 Mark]
Ans. A
Sol. Interior angle sum $=(n-2) \times 180^{\circ}$
Each angle of regular Hexagon.

$$
\begin{aligned}
& \frac{(n-2) \times 180^{\circ}}{n}=120^{\circ} \\
& \text { Required area }=\frac{\theta}{360^{\circ}} \times \pi \times 25=\frac{25 \pi}{3}
\end{aligned}
$$

4. A duck named Donald Duck says "All ducks always lie."

Based only on the information above, which one of the following statements can be logically inferred with certainty?
A. Donald Duck always lies.
B. Donald Duck always tells the truth.
C. Donald Duck's statement is true.
D. Donald Duck's statement is false.

## Ans. D

5. A line of symmetry is defined as a line that divides a figure into two parts in a way such that each part is a mirror image of the other part about that line.
The figure below consists of 20 unit squares arranged as shown. In addition to the given black squares, upto 5 more may be coloured black. Which one among the following options depicts the minimum number of boxes that must be coloured black to achieve two lines of symmetry? (The figure is representative)

A. d
B. $c, d, i$
C. $\mathrm{c}, \mathrm{i}$
D. $c, d, i, f, g$
[MCQ - 1 Mark]
Ans. B
Sol. Both $c, d, i$ and $C, d, i, f, g$ makes the figure symmetrical about two lines, but as minimum number is asked option (b) is correct.
6. Based only on the truth of the statement 'Some humans are intelligent', which one of the following options can be logically inferred with certainty?
A. No human is intelligent
B. All humans are intelligent
C. Some non-humans are intelligent
D. Some intelligent beings are humans
[MCQ - 2 Marks]
Ans. D
Sol.

Therefore, Option D is correct

7. Which one of the options can be inferred about the mean, median, and mode for the given probability distribution (i.e, probability mass function), $P(x)$, of a variable $x$ ?

A. mean $=$ median $\neq$ mode
B. mean $=$ median $=$ mode
C. mean $\neq$ median $=$ mode
D. mean $\neq$ mode $=$ median
[MCQ - 2 Marks]
Ans. A
8. The James Webb telescope, recently launched in space, is giving humankind unprecedented access to the depths of time by imaging very old stars formed almost 13 billion years ago. Astrophysicists and cosmologists believe that this odyssey in space may even shed light on the existence of dark matter. Dark matter is supposed to interact only via the gravitational interaction and not through the electromagnetic-, the weak- or the strong-interaction. This may justify the epithet "dark" in dark matter.
Based on the above paragraph, which one of the following statements is FALSE?
A. No other telescope has captured images of stars older than those captured by the James Webb telescope.
B. People other than astrophysicists and cosmologists may also believe in the existence of dark matter
C. The James Webb telescope could be of use in the research on dark matter
D. If dark matter was known to interact via the strong-interaction, then the epithet "dark" would be justified.
[MCQ - 2 Marks]
Ans. D
9. Let $a=30!, b=50$ !, and $C=100$ !. Consider the following numbers:
$\log _{\mathrm{a}} \mathrm{C} \quad \log _{\mathrm{c}} \mathrm{a}, \quad \log _{\mathrm{b}} a, \quad \log _{\mathrm{a}} b$
Which one of the following inequalities is CORRECT?
A. $\log _{\mathrm{c}} a<\log _{b} a<\log _{a} b<\log _{a} C$
B. $\log _{\mathrm{c}} a<\log _{\mathrm{a}} b<\log _{\mathrm{b}} a<\log _{\mathrm{b}} \mathrm{C}$
C. $\log _{\mathrm{c}} a<\log _{\mathrm{b}} a<\log _{a} \mathrm{C}<\log _{a} b$
D. $\log _{b} a<\log _{c} a<\log _{a} b<\log _{a} C$
[MCQ - 2 Marks]
Ans. A
Sol.

$$
\begin{aligned}
& w=\log _{a} c=\frac{\log c}{\log a}=\frac{\log 100!}{\log 30!} \\
& x=\log _{c} a=\frac{\log a}{\log c}=\frac{\log 30!}{\log 100!} \\
& y=\log _{a} b=\frac{\log b}{\log a}=\frac{\log 50!}{\log 30!} \\
& z=\log _{b} a=\frac{\log a}{\log b}=\frac{\log 30!}{\log 50!}
\end{aligned}
$$

clearly

$$
\begin{aligned}
& x<z<y<w \\
& \log _{c} a<\log _{b} a<\log _{a} b<\log _{a} c
\end{aligned}
$$

10. A square of side length 4 cm is given. The boundary of the shaded region is defined by one semi-circle on the top and two circular arcs at the bottom, each of radius 2 cm , as shown.
The area of the shaded region is $\qquad$ $\mathrm{cm}^{2}$.

A. 8
B. 4
C. 12
D. 10

Ans. A

## Sol.



$$
\begin{aligned}
\text { Total } & =2 \pi+(8-2 \pi) \\
& =8
\end{aligned}
$$

## Civil Engineering

11. For the integral $I=\int_{-1}^{1} \frac{1}{x^{2}} d x$ which of the following statements is TRUE?
A. $I=0$
B. $I=2$
C. $I=-2$
D. The integral does not change
[MCQ - 1 Mark]
Ans. D

Sol.

$$
\begin{aligned}
& \int_{-1}^{1} \frac{1}{x^{2}} \mathrm{dx} \rightarrow \text { Improper Integral of } 2^{\text {nd }} \text { Kind at } \mathrm{x}=0 \\
& \frac{1}{\mathrm{x}^{2}} \text { is discontinuous } \\
& \mathrm{I}=\int_{-1}^{1} \frac{1}{\mathrm{x}^{2}} \mathrm{dx}=\int_{-1}^{0} \frac{1}{\mathrm{x}^{2}} \mathrm{dx}+\int_{0}^{1} \frac{1}{\mathrm{x}^{2}} \mathrm{dx}=\mathrm{I}_{1}+\mathrm{I}_{2}
\end{aligned}
$$

$\rightarrow$ It is convergent if both integrals are convergent.

$$
\begin{aligned}
& \int_{0}^{1} \frac{1}{x^{2}} \mathrm{dx}=\left[-\frac{1}{\mathrm{x}}\right]_{0}^{1} \\
& =-\left[\frac{1}{1}-\frac{1}{0}\right] \\
& =-(1-\infty) \\
& =\text { undefined }
\end{aligned}
$$

so, $I$ is divergent.
12. A hanger is made of two bars of different sizes. Each bar has a square cross-section. The hanger is loaded by three-point loads in the mid vertical plane as shown in the figure. Ignore the self-weight of the hanger. What is the maximum tensile stress in $\mathrm{N} / \mathrm{mm}^{2}$ anywhere in the hanger without considering stress concentration effects?

A. 15.0
B. 25.0
C. 35.0
D. 45.0

Ans. B
Sol. For a bar of 100 mm square cross-section $A=100 \times 100$

$$
\begin{aligned}
& P=100+100+50=250 \mathrm{kN} \\
& \sigma=\frac{P}{A}=\frac{250 \times 10^{3}}{100 \times 100}=25 \mathrm{~N} / \mathrm{mm}^{2}
\end{aligned}
$$

For a bar of 50 mm square cross-section

$$
\begin{aligned}
& A=50 \times 50 \\
& P=50 \mathrm{kN} \\
& \sigma=\frac{P}{A}=\frac{50 \times 10^{3}}{50 \times 50}=20 \mathrm{~N} / \mathrm{mm}^{2}
\end{aligned}
$$

The maximum tensile stress is $25 \mathrm{~N} / \mathrm{mm}^{2}$
13. Creep of concrete under compression is defined as the
A. increase in the magnitude of strain under constant stress
B. increase in the magnitude of stress under constant strain
C. decrease in the magnitude of strain under constant stress
D. decrease in the magnitude of stress under constant strain
[MCQ - 1 Mark]
Ans. A
Sol. Creep under compression is defined as increase in the magnitude of strain under constant stress.
14. A singly reinforced concrete beam of balanced section is made of M20 grade concrete and Fe415 grade steel bars. The magnitudes of the maximum compressive strain in concrete and the tensile strain in the bars at ultimate state under flexure, as per IS 456: 2000 are, respectively. (Round off to four decimal places)
A. 0.0035 and 0.0038
B. 0.0020 and 0.0018
C. 0.0035 and 0.0041
D. 0.0020 and 0.0031
[MCQ - 1 Mark]
Ans. A
Sol. For M20, Fe415
As per IS 456, the maximum compressive strain in concrete $=0.0035$
Maximum compressive strain in steel

We know

$$
\mathrm{E}_{\mathrm{s}}=2 \times 10^{5} \mathrm{MPa}
$$

Maximum compressive strain in steel

$$
=0.002+\frac{0.87 \times 415}{2 \times 10^{5}}=0.0038
$$

15. In cement concrete mix design, with the increase in water-cement ratio, which one of the following statements is TRUE?
A. Compressive strength decreases but workability increases
B. Compressive strength increases but workability decreases
C. Both compressive strength and workability decrease
D. Both compressive strength and workability increase
[MCQ - 1 Mark]
Ans. A
Sol. Effect of water-cement ratio on the compressive strength of concreteAs the water-cement ratio increases, the compressive strength decreases.


Effect of water-cement ratio on the workability of concrete-
As the water-cement ratio increases, the workability of concrete decreases.
16. The specific gravity of a soil is 2.60 . The soil is at $50 \%$ degree of saturation with a water content of $15 \%$. The void ratio of the soil is.
A. 0.35
B. 0.78
C. 0.87
D. 1.28
[MCQ - 1 Mark]
Ans. B

## Sol.

$$
\begin{aligned}
& S=0.5 \\
& G=2.60 \\
& W=0.15
\end{aligned}
$$

As we know,

$$
\begin{aligned}
& \mathrm{Se}=\mathrm{WG} \\
& 0.5 \times \mathrm{e}=0.15 \times 2.60 \\
& \mathrm{e}=0.78
\end{aligned}
$$

17. A group of 9 friction piles are arranged in a square grid maintaining equal spacing in all directions. Each pile is of diameter 300 mm and length 7 m . Assume that the soil is cohesionless with effective friction angle $\phi^{\prime}=32^{\circ}$. What is the center-to-center spacing of the piles (in m ) for the pile group efficiency of $60 \%$ ?
A. 0.582
B. 0.486
C. 0.391
D. 0.677

## [MCQ - 1 Mark]

Ans. A or B

## Sol.



$$
\eta_{g}=\left(1-\frac{\theta}{90}\left[\frac{m(n-1)+n(m-1)}{m n}\right]\right) \times 100
$$

$$
0.6=\left(1-\frac{\theta}{90}\left[\frac{3(3-1)+3(3-1)}{3 \times 3}\right]\right) \times 100
$$

$$
\theta=27^{\circ}
$$

$$
\theta=\tan ^{3}\left(\frac{d}{s}\right)
$$

$$
27=\tan ^{-1}\left(\frac{0.3}{\mathrm{~S}}\right)
$$

$$
\mathrm{S}=0.587 \mathrm{~m}
$$

## Alternate Solution



$$
\begin{aligned}
& 0.6=\frac{(2 S+d) \times 4}{9 \times \pi d} \\
& S=\frac{1}{2}\left[\frac{0.6 \times 9 \times \pi \times 0.3}{4}-0.3\right]=0.486
\end{aligned}
$$

18. A possible slope failure is shown in the figure. Three soil samples are taken from different locations (I, II and III) of the potential failure plane. Which is the most appropriate shear strength test for each of the sample to identify the failure mechanism?
Identify the correct combination from the following options:
P: Triaxial compression test
Q: Triaxial extension test
R: Direct shear or shear box test
S: Vane shear test

A. I-Q, II-R, III-P
B. I-R, II-P, III-Q
C. I-S, II-Q, III-R
D. I-P, II-R, III-Q

Ans. A
Sol.

I. $\sigma_{H}>\sigma_{V} \rightarrow$ Triaxial extension (As $\sigma_{v}$ decreases depth from ground level is less)
II. Failure plane is same as direct shear box test i.e perpendicular to vertical plane
III. $\sigma_{v}>\sigma_{h} \rightarrow$ Triaxial compression test
19. When a supercritical stream enters a mild-sloped ( $M$ ) channel section, the type of flow profile would become $\qquad$ .
A. $M_{1}$
B. $\mathrm{M}_{2}$
C. $M_{3}$
D. $M_{1}$ and $M_{2}$
[MCQ - 1 Mark]
Ans. C
Sol.


When a super-critical steam enters a mild slope ( $M$ ) channel section, type of flow profile will be $M_{3}$.
20. Which one of the following statements is TRUE for Greenhouse Gas (GHG) in the atmosphere?
A. GHG absorbs the incoming short wavelength solar radiation to the earth surface, and allows the long wavelength radiation coming from the earth surface to pass through
B. GHG allows the incoming long wavelength solar radiation to pass through to the earth surface, and absorbs the short wavelength radiation coming from the earth surface
C. GHG allows the incoming long wavelength solar radiation to pass through to the earth surface, and allows the short wavelength radiation coming from the earth surface to pass through
D. GHG allows the incoming short wavelength solar radiation to pass through to the earth surface, and absorbs the long wavelength radiation coming from the earth surface

Ans. D
Sol. GHG allows the incoming short wavelength solar radiation to pass through to the earth surface and absorbs the long wavelength radiation coming from the earth surface.
21. $\mathrm{G}_{1}$ and $\mathrm{G}_{2}$ are the slopes of the approach and departure grades of a vertical curve, respectively. Given $\left|G_{1}\right|<\left|G_{2}\right|$ and $\left|G_{1}\right| \neq\left|G_{2}\right| \neq 0$
Statement 1: $+G_{1}$ followed by $+G_{2}$ results in a sag vertical curve.
Statement 2: $-G_{1}$ followed by $-G_{2}$ results in a sag vertical curve.
Statement 3: $+\mathrm{G}_{1}$ followed by $-\mathrm{G}_{2}$ results in a crest vertical curve.
Which option amongst the following is true?
A. Statement 1 and Statement 3 are correct; Statement 2 is wrong
B. Statement 1 and Statement 2 are correct; Statement 3 is wrong
C. Statement 1 is correct; Statement 2 and Statement 3 are wrong
D. Statement 2 is correct; Statement 1 and Statement 3 are wrong

Ans. A
Sol. Conditions for summit curve


Only statement 1 and 3 will fulfil the desired condition
22. The direct and reversed zenith angles observed by a theodolite are $56^{\circ} 00^{\prime} 00^{\prime \prime}$ and $303^{\circ} 00^{\prime} 00^{\prime \prime}$, respectively. What is the vertical collimation correction?
A. $+1^{\circ} 00^{\prime} 00^{\prime \prime}$
B. $-1^{\circ} 00^{\prime} 00^{\prime \prime}$
C. $-0^{\circ} 30^{\prime} 00^{\prime \prime}$
D. $+0^{\circ} 30^{\prime} 00^{\prime \prime}$

Ans. D
Sol. Direct zenith angle
Reversed zenith angle
Vertical collimation Error

$$
=56^{\circ}
$$

$$
=303^{\circ}
$$

$$
=\frac{360^{\circ}-(\text { sum of direct and reversed zenith angle })}{2}
$$

$$
=\frac{360^{\circ}-\left(56^{\circ}+303^{\circ}\right)}{2}
$$

$$
=0.5^{\circ}=30^{\prime}
$$

23. A student is scanning his 10 inch $\times 10$ inch certificate at 600 dots per inch (dpi) to convert it to raster. What is the percentage reduction in number of pixels if the same certificate is scanned at 300 dpi?
A. 62
B. 88
C. 75
D. 50
[MCQ - 1 Mark]
Ans. C
Sol.

$1 \mathrm{dpi}=12$ pixel $/ \mathrm{ft}$

$$
\Rightarrow 600 \mathrm{dpi}=(600 \times 12)
$$

For
10 inch $\times 10$ inch,
We get
$(600 \times 12)^{2}$
Similarly, for 300 dpi , we get $(300 \times 12)^{2}$
$\%$ Reduction in pixels $=\frac{(600 \times 12)^{2}-(300 \times 12)^{2}}{\left(600 \times 12^{2}\right)} \times 100=75 \%$
24. If $M$ is an arbitrary real $n \times n$ matrix, then which of the following matrices will have non-negative eigenvalues?
A. $M^{2}$
B. $M M^{\top}$
C. $M^{\top} M$
D. $\left(M^{\top}\right)^{2}$
[MSQ - 1 Mark]
Ans. A, B, C, D
25. The following function is defined over the interval $[-L, L]$ :

$$
f(x)=p x^{4}+q x^{3}
$$

If it is expressed as a Fourier series,

$$
F(x)=a_{0}+\sum_{n=1}^{\infty}\left\{a_{n} \sin \left(\frac{\pi x}{L}\right)+b_{n} \cos \left(\frac{\pi x}{L}\right)\right\}
$$

which options amongst the following are true?
A. $a_{n}, n=1,2, \cdots, \infty$ depend on $p$
B. $a_{n}, n=1,2, \cdots, \infty$ depend on $q$
C. $b_{n}, n=1,2, \cdots, \infty$ depend on $p$
D. $b_{n}, n=1,2, \cdots, \infty$ depend on $q$

Ans. B,C

## Sol.

$$
\begin{aligned}
& \mathrm{b}_{\mathrm{n}}=\frac{1}{\mathrm{~L}} \int_{-\mathrm{L}}^{\mathrm{L}} \mathrm{f}(\mathrm{x}) \cos \frac{(\mathrm{n} \pi \mathrm{x})}{\mathrm{L}} \mathrm{dx} \\
& =\frac{1}{\mathrm{~L}} \int_{-L}^{L} p x^{4} \cos \frac{(\mathrm{n} \pi x)}{\mathrm{L}} d x+0 \\
& \left(\mathrm{q} x^{5} \cos \frac{n \pi x}{\mathrm{~L}} \text { is odd function }\right)
\end{aligned}
$$

$\therefore \mathrm{b}_{\mathrm{n}}$ depends on p .

$$
\begin{aligned}
& a_{n}=\frac{1}{L} \int_{-L}^{L} f(x) \sin \frac{(n \pi x)}{L} d x \\
& =0+\frac{1}{L} \int_{-L}^{L} q x^{5} \sin \frac{(n \pi x)}{L} d x \\
& \left(p x^{4} \sin \frac{n \pi x}{L} \text { is odd function }\right)
\end{aligned}
$$

$\therefore \mathrm{a}_{\mathrm{n}}$ depends on q .
26. Consider the following three structures:


Which of the following statements is/are TRUE?
A. Structure I is unstable
B. Structure II is unstable
C. Structure III is unstable
D. All three structures are stable
[MSQ - 1 Mark]
Ans. A, B and C

## Sol. Structure I $\rightarrow$ Unstable

Internal hinge at $D$ can cause rigid body rotation.

## Structure II $\rightarrow$ Unstable

Since all reactions are concurrent at point A, rigid body rotation can take place.

## Structure III $\rightarrow$ Unstable

Even if $m=2 j-3$ criteria is satisfied, there is an improper arrangement of members in $1^{\text {st }}$ panel. Hence, shear in $1^{\text {st }}$ panel is not resisted by any member. So, it is unstable.
27. Identify the waterborne diseases caused by viral pathogens
A. Acute anterior poliomyelitis
B. Cholera
C. Infectious hepatitis
D. Typhoid fever
[MSQ - 1 Mark]
Ans. A, C
Sol. Waterborne diseases caused by Viral pathogens are-

1. Acute anterior poliomyelitis
2. Infectious hepatitis

Waterborne diseases caused by Bacterial pathogens are-

1. Cholera
2. Typhoid fever
3. Which of the following statements is/are TRUE for the Refuse-Derived Fuel (RDF) in the context of Municipal Solid Waste (MSW) management?
A. Higher Heating Value (HHV) of the unprocessed MSW is higher than the HHV of RDF processed from the same MSW
B. RDF can be made in the powdered form
C. Inorganic fraction of MSW is mostly converted to RDF
D. RDF cannot be used in conjunction with oil

Ans. B
Sol. A major inorganic fraction of Municipal solid waste can be converted into refuse-derived fuel (RDF) but not all of it. The process involves sorting, shredding, and compacting the inorganic waste materials into fuel pellets or briquettes. These RDF pellets can be burned as a fuel source in industries, such as cement and power plants, as a substitute for fossil fuels. The conversion of inorganic waste into RDF helps to reduce waste going to landfills, reduces greenhouse gas emissions, and provides a more sustainable energy source.
The HHV of RDF is generally higher than HVV of unprocessed MSW.
29. The probabilities of occurrences of two independent events $A$ and $B$ are 0.5 and 0.8 , respectively. What is the probability of occurrence of at least $A$ or $B$ (rounded off to one decimal place)? $\qquad$
[NAT - 1 Mark]
Ans. 0.9
Sol. P (at least one of A or B)

$$
\begin{aligned}
& P(A \cup B)=p(A)+p(B)-p(A \cap B) \\
& =p(A)+p(B)-p(A) p(B) \\
& =0.5+0.8-0.5 \times 0.8 \\
& =0.9
\end{aligned}
$$

30. In the differential equation $\frac{d y}{d x}+a x y=0$ positive constant. If $y=1.0$ at $x=0.0$, and $y=0.8$ at $x=1.0$, the value of $a$ is $\qquad$ (rounded off to three decimal places).
[NAT - 1 Mark]
Ans. 0.446
Sol.

$$
\begin{aligned}
& \frac{d y}{d x}=-\alpha x y \\
& \frac{d y}{y}=-\alpha x d x \\
& \log _{e} y=-\alpha \frac{x^{2}}{2}+\log _{e} c
\end{aligned}
$$

$$
\begin{aligned}
& \frac{y}{c}=e^{-\frac{\alpha x^{2}}{2}} \\
& y=c e^{-\alpha \frac{x^{2}}{2}} \ldots(1 \\
& y(0)=1 \\
& 1+1=c \Rightarrow c=1 \\
& \& y(1)=0.8 \\
& (1) \Rightarrow 0.8=1 \cdot e^{-\frac{\alpha}{2}} \\
& \Rightarrow \log _{e} 0.8=\frac{-\alpha}{2} \\
& \alpha=-2 \log _{e} 0.8=0.4463
\end{aligned}
$$

31. Consider the fillet-welded lap joint shown in the figure (not to scale). The length of the weld shown is the effective length. The welded surfaces meet at right angle. The weld size is 8 mm , and the permissible stress in the weld is 120 MPa . What is the safe load P (in kN , rounded off to one decimal place) that can be transmitted by this welded joint?

[NAT - 1 Mark]
Ans. 133 to 136
Sol.


Given,
Size of weld
Permissible stress

$$
\begin{aligned}
& =\mathrm{S}=8 \mathrm{~mm} \\
& =120 \mathrm{MPa} \\
& =0.7 \mathrm{~S}=0.7 \times 8=5.6 \mathrm{~mm} \\
& =l_{\text {eff. }}=75+50+75=200 \mathrm{~mm}
\end{aligned}
$$

Effective throat thickness
Effective length
Hence, safe load $=P=$ Permissible stress $\times$ Effective throat thickness $\times$ Effective length of weld

$$
\begin{aligned}
& =120 \times 5.6 \times 200 \\
& =134400 \mathrm{~N} \\
& =134.4 \mathrm{kN}
\end{aligned}
$$

32. A drained direct shear test was carried out on a sandy soil. Under a normal stress of 50 kPa , the test specimen failed at a shear stress of 35 kPa . The angle of internal friction of the sample is degree (round off to the nearest integer).

Ans. $35^{\circ}$
Sol.

$$
\begin{aligned}
& \sigma_{\mathrm{n}}=50 \mathrm{kPa} \\
& \tau=35 \mathrm{kPa}, \mathrm{C}=0 \\
& \tau=\mathrm{C}+\sigma_{\mathrm{n}} \tan \phi \\
& 35=0+50 \tan \phi \\
& \tan \phi=\frac{35}{50} \\
& \phi \cong 35^{\circ}
\end{aligned}
$$

33. A canal supplies water to an area growing wheat over 100 hectares. The duration between the first and last watering is 120 days, and the total depth of water required by the crop is 35 cm . The most intense watering is required over a period of 30 days and requires a total depth of water equal to 12 cm . Assuming precipitation to be negligible and neglecting all losses, the minimum discharge (in $\mathrm{m}^{3} / \mathrm{s}$, rounded off to three decimal places) in the canal to satisfy the crop requirement is $\qquad$ .
[NAT - 1 Mark]
Ans. 0.046
Sol. Area

$$
\begin{aligned}
& =A=100 \mathrm{Ha} \\
& =B=120 \text { days }
\end{aligned}
$$

$$
\text { Delta } \quad=\Delta=35 \mathrm{~cm}
$$

More intense watering is required for $=30$ days and the depth of water required (Delta for 30 days)

$$
=12 \mathrm{~cm}
$$

So, 12 cm is required in the first 30 days,
Hence, $(35-12)=23 \mathrm{~cm}$ will be required for $(120-30)=90$ days (next 90 days)
So, Discharge for the first 30 days:

$$
=\frac{\mathrm{A}}{\Delta}=\frac{\mathrm{A}}{864 \frac{\mathrm{~B}}{\Delta}}=\frac{100}{864 \times \frac{30}{12}}=0.046 \text { cumecs }
$$

Discharge for the next 90 days:

$$
=\frac{\mathrm{A}}{\Delta}=\frac{\mathrm{A}}{864 \frac{\mathrm{~B}}{\Delta}}=\frac{100}{864 \times \frac{90}{23}}=0.029 \text { cumecs }
$$

Hence, minimum design discharge required $=$ Max. $(0.046,0.029)$

$$
=0.046 \text { cumecs }
$$

## Note:

If we chose 0.029 cumecs, then the discharge requirement for the first 30 days will not be satisfied.
34. The ordinates of a one-hour unit hydrograph for a catchment are given below:

| t (hour) | 0 | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathrm{Q}\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | 0 | 9 | 21 | 18 | 12 | 5 | 2 | 0 |

Using the principle of superposition, a D-hour unit hydrograph for the catchment was derived from this one-hour unit hydrograph. The ordinates of the D -hour unit hydrograph were obtained as $3 \mathrm{~m}^{3} / \mathrm{s}$ at $\mathrm{t}=1$ hour and $10 \mathrm{~m}^{3} / \mathrm{s}$ at $\mathrm{t}=2$ hour. The value of D (in integer) is $\qquad$ _.
[NAT - 1 Mark]
Ans. 3

## Sol.

| Time | $\mathbf{Q}$ | S-curve addition | S-Curve ordinate | S-curve lagged (3 hr) | $\boldsymbol{\Delta y} \times\left(\frac{1}{\Delta T}\right)$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0 | - | 0 | - | 0 |
| 1 | 9 | 0 | 9 | - | $(9-0) \times \frac{1}{3}=3 \mathrm{~m}^{3} / \mathrm{sec}$ |
| 2 | 21 | 9 | 30 | 0 | $(30-0) \times \frac{1}{3}=10 \mathrm{~m}^{3} / \mathrm{sec}$ |
| 3 | 18 | 30 | 48 | 9 |  |
| 4 | 12 | 48 | 60 | 48 |  |
| 5 | 5 | 60 | 65 | 67 |  |
| 6 | 2 | $\Delta T=3$ |  |  |  |
| $\therefore$ |  |  |  |  |  |

35. For a horizontal curve, the radius of a circular curve is obtained as 300 m with the design speed as 15 $\mathrm{m} / \mathrm{s}$. If the allowable jerk is $0.75 \mathrm{~m} / \mathrm{s}^{3}$, what is the minimum length (in m , in integer) of the transition curve $\qquad$ ?

Ans. 15
Sol. Given,

$$
\begin{aligned}
& \mathrm{R}=300 \mathrm{~m} \\
& \mathrm{~V}=15 \mathrm{~m} / \mathrm{sec} \\
& \mathrm{C}=0.75 \mathrm{~m} / \mathrm{sec}^{2} \\
& \mathrm{~L}_{\mathrm{s}}=\text { Length of transition curve } \\
& \mathrm{L}_{\mathrm{s}}=\frac{\mathrm{V}^{3}}{\mathrm{CR}}=\frac{15^{3}}{0.75 \times 300}=15 \mathrm{~m}
\end{aligned}
$$

Note: Since we don't know anything about the terrain hence, We cannot find out the length by formula given by IRC
36. A function ( $x$ ), that is smooth and convex shaped between interval ( $x_{1}, x_{u}$ ) is shown in the figure. This function is observed at odd number of regularly spaced points. If the area under the function is computed numerically, then $\qquad$ .

A. The numerical value of the area obtained using the trapezoidal rule will be less than the actual
B. The numerical value of the area obtained using the trapezoidal rule will be more than the actual
C. The numerical value of the area obtained using the trapezoidal rule will be exactly equal to the actual
D. With the given details, the numerical value of area cannot be obtained using trapezoidal rule
[MCQ - 2 Marks]
Ans. A
37. Consider a doubly reinforced RCC beam with the option of using either Fe 250 plain bars or Fe 500 deformed bars in the compression zone. The modulus of elasticity of steel is $2 \times 10^{5} \mathrm{~N} / \mathrm{mm}^{2}$. As per IS 456:2000, in which types) of the bars, the stress in the compression steel (c) can reach the design strength ( $0.87 \mathrm{f}_{\mathrm{y}}$ ) at the limit state of collapse?
A. Fe250 plain bars only
B. Fe500 deformed bars only
C. Both Fe250 plain bars and Fe500 deformed bars
D. Neither Fe250 plain bars nor Fe500 deformed bars

Ans. A

## Sol.



So,

$$
\mathrm{E}_{\mathrm{sc}}<0.0035
$$

$$
\mathrm{E}_{\mathrm{sc}}=0.0035 \times \frac{\left(\mathrm{x}_{\mathrm{u}}-\mathrm{d}_{\mathrm{c}}\right)}{\mathrm{x}_{\mathrm{u}}}
$$

But we can consider

$$
E_{s c} \approx 0.0035 .
$$

For Fe 250

The permissible strain is

$$
\frac{0.87 f_{y}}{\epsilon_{s}}=\frac{0.87 \times 250}{2 \times 10^{5}}=1.0875 \times 10^{-3}
$$



When

$$
E_{s c}=0.0035 \quad f_{s c}=0.87 f_{y}
$$

For Fe500

The permissible strain

$$
\begin{aligned}
& =\frac{0.87 f_{y}}{\epsilon_{s}}+0.002 \\
& =\frac{0.87 \times 500}{2 \times 10^{5}}+0.002 \\
& =4.175 \times 10^{-3}
\end{aligned}
$$



Hence,

$$
f_{s c}<0.87 f_{y}
$$

Fe250 will reach to $0.87 f_{y}$
38. Consider the horizontal axis passing through the centroid of the steel beam cross-section shown in the figure. What is the shape factor (rounded off to one decimal place) for the cross-section?

A. 1.5
B. 1.7
C. 1.3
D. 2.0

Ans. B
Sol.


Plastic Section modulus:

$$
\begin{aligned}
& \bar{y}_{1}=\frac{(3 b)\left(\frac{b}{2}\right)\left(\frac{b}{4}\right)+(b)(b)\left(\frac{b}{2}+\frac{b}{2}\right)}{(3 b)\left(\frac{b}{2}\right)+(b)(b)} \\
& \bar{y}_{1}=\frac{\frac{3 b^{3}}{8}+b^{3}}{\frac{3 b^{2}}{2}+b^{2}} \\
& \bar{y}_{1}=\frac{1.375 b^{3}}{2.5 b^{2}}=0.55 b
\end{aligned}
$$

Elastic section modulus:

$$
Z_{p}=\frac{A}{2}\left(\bar{y}_{1}+\bar{y}_{2}\right)=\frac{5 b^{2}}{2}(0.55 b+0.55 b)=2.75 b^{3}
$$

$$
\begin{aligned}
& \mathrm{Z}=\frac{\mathrm{I}}{\mathrm{Y}_{\max }}=\frac{\frac{(\mathrm{b})(3 \mathrm{~b})^{3}}{12}+\frac{(2 \mathrm{~b})(\mathrm{b})^{3}}{12}}{3 \mathrm{~b} / 2} \\
& \mathrm{Z}_{\mathrm{l}}=1.611 \mathrm{~b}^{3} \\
& \text { Shape factor }=\frac{\mathrm{Z}_{\mathrm{p}}}{\mathrm{Z}_{\mathrm{e}}}=\frac{2.75 \mathrm{~b}^{3}}{1.611 \mathrm{~b}^{3}}=1.7
\end{aligned}
$$

39. Consider the pin-jointed truss shown in the figure (not to scale). All members have the same axial rigidity, AE. Members QR, RS, and ST have the same length $L$. Angles QBT, RCT, SDT are all $90^{\circ}$. Angles BQT, CRT, DST are all $30^{\circ}$. The joint $T$ carries a vertical load $P$. The vertical deflection of joint $T$ is $k \frac{P L}{A E}$. What is the value of $k$ ?

A. 1.5
B. 4.5
C. 3.0
D. 9.0

Ans. B
Sol. Member SD, SC, CR, RB will be zero force members. (At a joint, if 3 members are meeting, and 2 members are collinear, then force in the third member is zero)
Member $B Q$ will also be zero since the reaction at $B$ and member $B T$ are collinear.
So, the structure reduces to


## For P-values of forces:

At joint T

$$
\Sigma F_{V}=0
$$

$F_{\text {TQ }} \sin 60=P$

$$
\begin{align*}
& \mathrm{F}_{\mathrm{TQ}}=\frac{2 \mathrm{P}}{\sqrt{3}}(\mathrm{~T}) \\
& \Sigma \mathrm{F}_{\mathrm{H}}=0 \\
& \mathrm{~F}_{\mathrm{TQ}} \cos 60=\mathrm{F}_{\mathrm{TB}} \\
& \frac{2 \mathrm{P}}{\sqrt{3}} \times \frac{1}{2}=\mathrm{F}_{\mathrm{TB}} \Rightarrow \mathrm{~F}_{\mathrm{TB}}=\frac{\mathrm{P}}{\sqrt{3}} \tag{C}
\end{align*}
$$

## For K-values of forces:

Apply a unit load at T in the downward direction.


At joint $T$

$\Sigma F_{V}=0$
$\mathrm{F}_{\mathrm{TQ}} \sin 60=1$

$$
\mathrm{F}_{\mathrm{TQ}}=\frac{2}{\sqrt{3}}(\mathrm{~T})
$$

$$
\Sigma \mathrm{F}_{\mathrm{H}}=0
$$

$\mathrm{F}_{\mathrm{TQ}} \cos 60=\mathrm{F}_{\mathrm{TB}}$
$\frac{2}{\sqrt{3}} \times \frac{1}{2}=F_{T B} \Rightarrow F_{T B}=\frac{1}{\sqrt{3}}$

| Member | P-value | K-value | Length | PKL/AE |
| :---: | :---: | :---: | :---: | :---: |
| TB | $-\frac{P}{\sqrt{3}}$ | $-\frac{1}{\sqrt{3}}$ | $\frac{3 \mathrm{~L}}{2}$ | $\frac{\mathrm{PL}}{2 \mathrm{AE}}$ |
| TQ | $\frac{2 \mathrm{P}}{\sqrt{3}}$ | $\frac{2}{\sqrt{3}}$ | 3 L | $\frac{4 \mathrm{PL}}{\mathrm{AE}}$ |

$$
\begin{aligned}
& \Delta_{\mathrm{VT}}=\sum \frac{\mathrm{PKL}}{\mathrm{AE}}=\frac{9 \mathrm{PL}}{2 \mathrm{AE}}=\frac{4.5 \mathrm{PL}}{\mathrm{AE}} \\
& \Rightarrow \mathrm{k}=4.5
\end{aligned}
$$

40. With reference to the compaction test conducted on soils, which of the following is INCORRECT?
A. Peak point of the compaction curve gives the maximum dry unit weight and optimum moisture content
B. With increase in the compaction effort, the maximum dry unit weight increases
C. With increase in the compaction effort, the optimum moisture content decreases
D. Compaction curve crosses the zero-air-voids curve
[MCQ - 2 Marks]
Ans. D

## Sol.

> Peak point of the compaction curve gives the maximum dry unit weight and optimum moisture content.
> With increase in the compaction effort, the maximum dry unit weight increases
$>$ With increase in the compaction effort, the optimum moisture content decreases

> Compaction curve do not cross the zero-air-voids line

41. Consider that a force $P$ is acting on the surface of a half-space (Bossiness's problem). The expression for the vertical stress $(\sigma z)$ at any point $(r, z)$, within the half-space is given as,

$$
\sigma_{z}=\frac{3 P}{2 \pi} \frac{z^{3}}{\left(r^{2}+z^{2}\right)^{\frac{5}{2}}}
$$

where, $r$ is the radial distance, and $z$ is the depth with downward direction taken as positive. At any given $r$, there is a variation of $\sigma_{z}$ along $z$, and at a specific $z$, the value of $\sigma_{z}$ will be maximum. What is the locus of the maximum $\sigma z$ ?
A. $z^{2}=\frac{3}{2} r^{2}$
B. $z^{3}=\frac{3}{2} r^{2}$
C. $z^{2}=\frac{5}{2} r^{2}$
D. $z^{3}=\frac{5}{2} r^{2}$

Ans. A

## Sol.

$$
\begin{gathered}
\sigma_{z}=\frac{3 P}{2 \pi} \frac{z^{3}}{\left(r^{2}+z^{2}\right)^{\frac{5}{2}}} \\
\frac{d \sigma_{z}}{d z}=0 \\
\frac{3 P}{2 \pi}\left[\frac{\left(r^{2}+z^{2}\right)^{\frac{5}{2}} 3 z^{2}-z^{3} \frac{5}{2}\left(r^{2}+z^{2}\right)^{\frac{3}{2}} 2 z}{\left(r^{2}+z^{2}\right)^{5}}\right]=0 \\
3 r^{2}+3 z^{2}-5 z^{2}=0 \\
z^{2}=\frac{3}{2} r^{2}
\end{gathered}
$$

42. A square footing of size $2.5 \mathrm{~m} \times 2.5 \mathrm{~m}$ is placed 1.0 m below the ground surface on a cohesionless homogeneous soil stratum. Considering that the groundwater table is located at the base of the footing, the unit weights of soil above and below the groundwater table are $18 \mathrm{kN} / \mathrm{m}^{3}$ and $20 \mathrm{kN} / \mathrm{m}^{3}$, respectively, and the bearing capacity factor $N q$ is 58 , the net ultimate bearing capacity of the soil is estimated as 1706 kPa (unit weight of water $=10 \mathrm{kN} / \mathrm{m}^{3}$ ).
Earlier, a plate load test was carried out with a circular plate of 30 cm diameter in the same foundation pit during a dry season, when the water table was located beyond the plate influence zone. Using Terzaghi's bearing capacity formulation, what is the ultimate bearing capacity (in KPa ) of the plate?
A. 110.16
B. 61.20
C. 204.00
D. 163.20
[MCQ - 2 Marks]
Ans. A
Sol.

$$
\begin{aligned}
& \mathrm{q}_{\mathrm{nu}}=\mathrm{q}_{\mathrm{u}}-\bar{\sigma} \\
& \mathrm{q}_{\mathrm{nu}}=\mathrm{q}_{\mathrm{u}}-\gamma \mathrm{D}_{\mathrm{f}} \\
& \mathrm{q}_{\mathrm{nu}}=\left\{1.3 C N_{\mathrm{c}}+\gamma \mathrm{D}_{\mathrm{f}} \mathrm{~N}_{\mathrm{q}}+0.4 \mathrm{~B} \gamma \mathrm{~N}_{\gamma}\right\}-\gamma \mathrm{D}_{\mathrm{f}}
\end{aligned}
$$

For cohesionless homogeneous soil stratum $\mathrm{C}=0$

$$
\begin{aligned}
& \mathrm{q}_{\mathrm{nu}}=\left\{\gamma \mathrm{D}_{\mathrm{f}} \mathrm{~N}_{\mathrm{q}}+0.4 \mathrm{~B} \gamma \mathrm{~N}_{\gamma}\right\}-\gamma \mathrm{D}_{\mathrm{f}} \\
& 1706=\left\{18 \times 1 \times 58+0.4 \times 2.5 \times 10 \times \mathrm{N}_{\gamma}\right\}-18 \\
& \mathrm{~N}_{\gamma}=68
\end{aligned}
$$

For circular Plate
For cohesionless homogeneous soil stratum $\mathrm{C}=0$ and depth of footing $=0$

$$
\mathrm{q}_{\mathrm{u}}=0.3 \times 0.3 \times 18 \times 68=110.16 \mathrm{kPa}
$$

Note: Surcharge at the plate level is zero
43. A very wide rectangular channel carries a discharge ( $Q$ ) of $70 \mathrm{~m}^{3} / \mathrm{s}$ per meter width. Its bed slope changes from 0.0001 to 0.0009 at a point $P$, as shown in the figure (not to scale). The Manning's roughness coefficient of the channel is 0.01 . What water surface profile(s) exist(s) near the point $P$ ?

A. $\mathrm{M}_{2}$ and $\mathrm{S}_{2}$
B. $M_{2}$ only
C. $\mathrm{S}_{2}$ Only
D. $\mathrm{S}_{2}$ and hydraulic jump

Ans. A

## Sol.



CDL will remain same.
But since, NDL varies inversely with slope, the NDL in mild slope is above the CDL, but in steep slope, NDL is below the CDL.
So, as clear from the diagram, flow profile will be $M_{2}-S_{2}$.
44. A jet of water having a velocity of $20 \mathrm{~m} / \mathrm{s}$ strikes a series of plates fixed radially on a wheel revolving in the same direction as the jet at $15 \mathrm{~m} / \mathrm{s}$. What is the percentage efficiency of the plates? (round off to one decimal place)
A. 37.5
B. 66.7
C. 50.0
D. 88.9
[MCQ - 2 Marks]
Ans. A
Sol. Efficiency of plates,

$$
\begin{aligned}
\eta & =\frac{2 u(V-u)}{V^{2}} \times 100 \\
u & =\text { velocity of plate }=15 \mathrm{~m} / \mathrm{s} \\
\mathrm{~V} & =\text { velocity of jet }=20 \mathrm{~m} / \mathrm{s} \\
\eta & =\frac{2 \times 15 \times(20-15)}{20^{2}} \times 100=37.5 \%
\end{aligned}
$$

45. In the following table, identify the correct set of associations between the entries in Column-1 and Column-2.
A. P-II, Q-I, S-III
C. P-IV, R-I, S-II
D. P-III, Q-I, R-IV

| Column-1 | Column-1 |
| :--- | :--- |
| P: Reverse Osmosis | I: Ponding |
| Q: Trickling Filter | II: Freundlich Isotherm |
| R: Coagulation | III: Concentration Polarization |
| S: Adsorption | IV: Charge Neutralization |

[MCQ - 2 Marks]
Ans. D
Sol. Reverse Osmosis - Concentration Polarization
Trickling Filter - Ponding
Coagulation- Charge Neutralization
Adsorption- Freundlich Isotherm
46. A plot of speed-density relationship (linear) of two roads (Road A and Road B) is shown in the figure.


If the capacity of Road $A$ is $C A$ and the capacity of Road $B$ is $C B$, what is $\frac{C_{A}}{C_{B}}$
A. $\frac{\mathrm{k}_{\mathrm{A}}}{\mathrm{k}_{\mathrm{B}}}$
B. $\frac{\mathrm{u}_{\mathrm{A}}}{\mathrm{u}_{\mathrm{B}}}$
C. $\frac{\mathrm{k}_{\mathrm{A}} \mathrm{u}_{\mathrm{A}}}{\mathrm{k}_{\mathrm{B}} \mathrm{u}_{\mathrm{B}}}$
D. $\frac{\mathrm{k}_{\mathrm{A}} \mathrm{u}_{\mathrm{B}}}{\mathrm{k}_{\mathrm{B}} \mathrm{u}_{\mathrm{A}}}$

Ans. C
Sol.


As per Greenshield model, we know.

$$
\mathrm{q}_{\max }=\frac{\mathrm{V}_{\mathrm{sf}} \mathrm{k}_{\mathrm{j}}}{4}
$$

$$
\frac{\text { Capcity } A}{\text { Capcity } B}=\frac{\frac{\left(\mathrm{k}_{\mathrm{J}}\right)_{\mathrm{A}} \times\left(\mathrm{V}_{\mathrm{SF}}\right)_{\mathrm{A}}}{4}}{\frac{\left(\mathrm{k}_{\mathrm{J}}\right)_{\mathrm{B}} \times\left(\mathrm{V}_{\mathrm{SF}}\right)_{\mathrm{B}}}{4}}=\frac{\mathrm{K}_{\mathrm{A}} \times \mathrm{U}_{\mathrm{A}}}{\mathrm{~K}_{\mathrm{B}} \times \mathrm{U}_{\mathrm{B}}}
$$

47. For the matrix

$$
[A]=\left[\begin{array}{lll}
1 & 2 & 3 \\
3 & 2 & 1 \\
3 & 1 & 2
\end{array}\right]
$$

which of the following statements is/are TRUE?
A. The eigenvalues of $[A]^{\top}$ are same as the eigenvalues of $[A]$
B. The eigenvalues of $[A]^{-1}$ are the reciprocals of the eigenvalues of $[A]$
C. The eigenvectors of $[A]^{\top}$ are same as the eigenvectors of [A]
D. The eigenvectors of $[A]^{-1}$ are same as the eigenvectors of $[A]$

Ans. A, B, D
Sol. By standard properties:
Eigen values of $[A]^{\top}$ and $[A]$ are same.
Eigen vectors of $[A]$ and $[A]^{\top}$ are not same.
Eigen values of $[A]^{-1}$ is reciprocal of Eigen value of $[A]$.
Eigen vectors of $[A]^{-1}$ are same as the eigenvectors of $[A]$
48. For the function $f(x)=e^{x}|\sin x| ; x \in R$, which of the following statements is/are TRUE?
A. The function is continuous at all $x$
B. The function is differentiable at all $x$
C. The function is periodic
D. The function is bounded

Ans. A
Sol. $y(x)=e^{x}|\sin x|$
$\mathrm{e}^{\mathrm{x}} \rightarrow$ Aperiodic \& unbounded


Consider,

$$
-\pi<\mathrm{X}<\pi
$$

$$
f(x)=\left\{\begin{array}{rr}
e^{x} \sin x, & 0<x<\pi \\
-e^{x} \sin x, & -\pi<x<0
\end{array}\right.
$$

At

$$
x=0,
$$

$$
\mathrm{LHL}=-\mathrm{e}^{0} \sin 0=0
$$

$$
R H L=e^{0} \sin 0=0
$$

$$
\mathrm{LHL}=\mathrm{RHL}=\mathrm{f}(0) \rightarrow \text { continuous }
$$

$$
f^{\prime}(x)=\left\{\begin{array}{rr}
e^{x} \cos +e^{x} \sin x, & 0<x<\pi \\
-e^{x} \cos x-e^{x} \sin x, & -\pi<x<0
\end{array}\right.
$$

At

$$
\begin{aligned}
& x=0 \\
& \text { LHD }=-e^{0} \times \cos 0-e^{0} \sin 0=-1 \\
& R H D=e^{0} \cos 0+e^{0} \sin 0=+1 \\
& \text { LHD } \neq \text { RHD } \rightarrow \text { Not differentiable } .
\end{aligned}
$$

49. Consider the beam shown in the figure (not to scale), on a hinge support at end $A$ and a roller support at end $B$. The beam has a constant flexural rigidity, and is subjected to the external moments of magnitude $M$ at one-third spans, as shown in the figure. Which of the following statements is/are TRUE?

A. Support reactions are zero
B. Shear force is zero everywhere
C. Bending moment is zero everywhere
D. Deflection is zero everywhere

Ans. A, B
Sol.

$\Sigma \mathrm{V}=0$
$\mathrm{R}_{\mathrm{A}}+\mathrm{R}_{\mathrm{B}}=0$
$\Sigma \mathrm{M}_{\mathrm{A}}=0$
$+R_{B} \times 3 L-M+M=0$
$\mathrm{R}_{\mathrm{B}}=0$
$R_{A}=0$

Since there is no vertical loading the shear force will also be zero anywhere on the beam.
Calculating bending moment:
Bending moment at span AC


From $A$ to left of $C$, no bending moment.
Bending moment at span CD


Bending moment of span DB


$$
\begin{gathered}
x \\
M_{x x}=-M+M=0 \\
M_{D}=0(\text { Right of } D) \\
M_{B}=0
\end{gathered}
$$

Shear force and bending moment diagram.


Deflection anywhere on the beam $=\frac{M}{E I}$ diagram $\times$ centroid.
Since the bending moment is not zero, so the deflection will exist.
50. Which of the following statements is/are TRUE in relation to the Maximum Mixing Depth (or Height) ' $D_{\max }$ ' in the atmosphere?
A. $D_{\text {max }}$ is always equal to the height of the layer of unstable air
B. Ventilation coefficient depends on $D_{\max }$
C. A smaller $D_{\max }$ will have a smaller air pollution potential if other meteorological conditions remain same
D. Vertical dispersion of pollutants occurs up to $D_{\text {max }}$

Ans. B and D

Sol. The ventilation coefficient depends on the maximum mixing depth. The ventilation coefficient is a measure of the rate of air exchange between indoor and outdoor environments. It represents the fraction of outdoor air that is brought into a building through natural or mechanical ventilation systems. The more the maximum mixing depth, the more complete the air exchange between indoor and outdoor air will be. Vertical dispersion refers to the vertical mixing of pollutants in the atmosphere. A smaller vertical dispersion means that pollutants are less likely to spread out vertically and become diluted in the air, leading to a higher concentration of pollutants at the ground level. This can lead to a higher potential for air pollution and negative impacts on human health and the environment. Therefore, smaller vertical dispersion is generally considered an indicator of a higher air pollution potential.
The maximum mixing depth is not always equal to half of the layer of unstable air. The maximum mixing depth refers to the height at which air is well mixed between the indoor and outdoor environments and is determined by several factors, including atmospheric stability, wind speed, and building design. The layer of unstable air is determined by the temperature and moisture profile of the atmosphere and refers to the height at which vertical air mixing occurs.
In certain atmospheric conditions, the maximum mixing depth may be equal to half of the layer of unstable air, but this is not always.
51. Which of the following options match the test reporting conventions with the given material tests in the table?

| Test reporting convention | Material test |
| :--- | :--- |
| (P) Reported as ratio | (I) Solubility of bitumen |
| (Q) Reported as percentage | (II) Softening point of bitumen |
| (R) Reported in temperature | (III) Los Angeles abrasion test |
| (S) Reported in length | (IV) Flash point of bitumen |
|  | (V) Ductility of bitumen |
|  | (VI) Specific gravity of bitumen |
|  | (VII) Thin film oven test |
| (Q) - (I); (R) -(II);(S)- (VII) | B. (P)-(VI);(Q)-(III); (R) - (IV); (S) - (V) |
| (Q)- (I); (R) - (II); (S) - (V) | D. (P)-(VI);(Q)-(III); (R)- (IV); (S) (VII) |

A. (P)-(VI); (Q)-(I); (R) -(II);(S)-(VII)
D. (P)-(VI);(Q)-(III); (R)- (IV); (S) (VII)
C. (P)-(VI); (Q)-(I); (R) - (II); (S) - (V)
[MSQ - 2 Marks]
Ans. B, C
Sol. Specific gravity of bitumen is the ratio of mass of given volume of substance to the mass of equal volume of water, the temperature of both being specified.
(P) - (VI);
(Q) - (I, III, VII);
(R) - (II, IV); (S) - (V)
52. The differential equation,

$$
\frac{\mathrm{du}}{\mathrm{dt}}+2 \mathrm{tu}^{2}=1
$$

is solved by employing a backward difference scheme within the finite difference framework. The value of u at the $(\mathrm{n}-1)^{\text {th }}$ time-step, for some n , is 1.75 . The corresponding time ( t ) is 3.14 s . Each time step is 0.01 s long. Then, the value of $\left(u_{n}-u_{n}-1\right)$ is (round off to three decimal places).
[NAT - 2 Marks]
Ans. -0.151
Sol.

$$
\begin{aligned}
& \frac{d u}{d t}=1-2 t u^{2} \\
& y_{n}=y_{n-1}+h_{f}\left(x_{n}, y_{n}\right) \\
& u_{n}=u_{n-1}+h f\left(t_{n}, u_{n}\right) \\
& u_{n-1}=1.75, t_{n-1}=3.14 \mathrm{~s}, \mathrm{~h}=0.0 \\
& t_{n}=3.14+0.01=3.15 \mathrm{~s} \\
& u_{n}=1.75+0.01\left[1-2 \times 3.15 u_{n}^{2}\right] \\
& u_{n}=1.75+0.01-0.063 u_{n}^{2}
\end{aligned}
$$

Given: $\quad u_{n-1}=1.75, t_{n-1}=3.14 \mathrm{~s}, \mathrm{~h}=0.01 \mathrm{~s}$
So,

$$
\begin{aligned}
& 0.063 u_{n}^{2}+u_{n}-1.76=0 \\
& u_{n}=1.5989 \checkmark \\
& u_{n}=-17.47 x \\
& u_{n}-u_{n-1}=-0.1511
\end{aligned}
$$

53. The infinitesimal element shown in the figure (not to scale) represents the state of stress at a point in a body. What is the magnitude of the maximum principal stress (in $\mathrm{N} / \mathrm{mm}^{2}$, in integer) at the point?

[NAT - 2 Marks]
Ans. 7
Sol.

$$
\sigma_{y y}=6 \mathrm{~N} / \mathrm{mm}^{2} \text { and } \tau_{x y}=3 \mathrm{~N} / \mathrm{mm}^{2}
$$

We know.

$$
\begin{aligned}
& \sigma_{\theta}=\frac{\sigma_{x x}+\sigma_{y y}}{2}+\frac{\sigma_{x x}-\sigma_{y y}}{2} \cos 2 \theta+\tau_{x y} \sin 2 \theta \\
& 5=\frac{\sigma_{x x}+6}{2}+\frac{\sigma_{x x}-6}{2} \cos 90+3 \sin 90^{\circ} \\
& \sigma_{x x}=-2 \mathrm{~N} / \mathrm{mm}^{2}
\end{aligned}
$$

Maximum Principal Stress

$$
\begin{aligned}
& \sigma_{1}=\frac{\sigma_{x x}+\sigma_{y y}}{2}+\sqrt{\left(\frac{\sigma_{x x}-\sigma_{y y}}{2}\right)^{2}+\tau_{x y}^{2}} \\
& =\frac{-2+6}{2}+\sqrt{\left(\frac{-2-6}{2}\right)^{2}+3^{2}} \\
& =2+\sqrt{4^{2}+3^{2}} \\
& =2+5 \\
& =7 \mathrm{~N} / \mathrm{mm}^{2}
\end{aligned}
$$

54. An idealised bridge truss is shown in the figure. The force in Member $U_{2} L_{3}$ is $\qquad$ kN (round off to one decimal place)

[NAT - 2 Marks]
Ans. 14.1

## Sol.



Vertical reaction at support at $U_{0}$ and $U_{6}$

$$
=\frac{20 \times 5}{2}=50 \mathrm{kN}
$$

Cut a section $x-x$ as shown in the figure and consider equilibrium of LHS.

55. The cross-section of a girder is shown in the figure (not to scale). The section is symmetric about a vertical axis ( $Y-Y$ ). The moment of inertia of the section about the horizontal axis ( $X-X$ ) passing through the centroid is $\mathrm{cm}^{4}$ (round off to nearest integer).

[NAT - 2 Marks]
Ans. 468775 to 468810

## Sol.



Taking $\bar{y}$ from top

$$
\begin{aligned}
& \bar{y}=\frac{A_{1} y_{1}+A_{2} y_{2}}{A_{1}+A_{2}} \\
& \bar{y}=\frac{40 \times 10 \times 5+50 \times 20 \times 35}{40 \times 10+50 \times 20}=26.428 \mathrm{~cm} \\
& I_{N A}=\frac{b d^{3}}{12}+A h^{2} \\
& I_{N A}=\frac{40 \times 10^{3}}{12}+40 \times 10 \times(26.428-5)^{2} \\
& +\frac{20 \times 50^{3}}{12}+50 \times 20 \times(26.428-10-25)^{2} \\
& \cong 468775.2 \mathrm{~cm}^{2}=468775 \mathrm{~cm}^{2}
\end{aligned}
$$

56. A soil having the average properties, bulk unit weight $=19 \mathrm{kN} / \mathrm{m}^{3}$; angle of internal friction $=25^{\circ}$ and cohesion $=15 \mathrm{kPa}$, is being formed on a rock slope existing at an inclination of $35^{\circ}$ with the horizontal. The critical height (in m ) of the soil formation up to which it would be stable without any failure is (round off to one decimal place).
[Assume the soil is being formed parallel to the rock bedding plane and there is no ground water effect.]
[NAT - 2 Marks]
Ans. 5.02 to 5.05
Sol.

$$
\begin{aligned}
& \mathrm{C}+\gamma \mathrm{H}_{\mathrm{C}} \cos ^{2} \beta \tan \phi=\gamma \mathrm{H}_{\mathrm{C}} \sin \beta \cos \beta \\
& 15+19 \mathrm{H}_{\mathrm{C}} \cos ^{2} 35 \tan 25=19 \mathrm{H}_{\mathrm{C}} \operatorname{Sin} 35 \cos 35 \\
& \mathrm{H}_{\mathrm{C}}=5.03 \mathrm{~m}
\end{aligned}
$$

57. A smooth vertical retaining wall supporting layered soils is shown in figure. According to Rankine's earth pressure theory, the lateral active earth pressure acting at the base of the wall is kPa (round off to one decimal place).

Surcharge load, $\mathrm{q}=20 \mathrm{kPa}$

[NAT - 2 Marks]
Ans. 35.3 to 35.8

## Sol.

$$
\begin{aligned}
& \quad \begin{array}{l}
\gamma_{\mathrm{b}}=20 \mathrm{kN} / \mathrm{m}^{3} \\
\mathrm{C}=0, \phi=32^{\circ} \\
\mathrm{C}=20 \mathrm{kN} / \mathrm{m}^{2}
\end{array} \\
& \sigma_{\mathrm{H}}=\mathrm{K}_{\mathrm{a}} \sigma_{\mathrm{v}}-2 \mathrm{C} \sqrt{\mathrm{~K}_{\mathrm{a}}} \\
& \mathrm{~K}_{\mathrm{a}}=\frac{1-\sin 25}{1+\sin 25}=0.408
\end{aligned} \sigma_{\mathrm{H}}=0.408[20+18 \times 3+19 \times 4]-2 \times 20 \times \sqrt{0.408}
$$

58. A vertical trench is excavated in a clayey soil deposit having a surcharge load of 30 kPa . A fluid of unit weight $12 \mathrm{kN} / \mathrm{m}^{3}$ is poured in the trench to prevent collapse as the excavation proceeds. Assume that the fluid is not seeping through the soil deposit. If the undrained cohesion of the clay deposit is 20 kPa and saturated unit weight is $18 \mathrm{kN} / \mathrm{m}^{3}$, what is the maximum depth of unsupported excavation (in m , rounded off to two decimal places) $\qquad$ ?
[NAT - 2 Marks]
Ans. 3.33
Sol.


Now, active pressure at depth z,

$$
\sigma_{z}=k_{a} \gamma z-2 C \sqrt{k_{a}}-12 z+k q
$$

At

At
$z=0$
$\left[\because \mathrm{k}=1\right.$ for $\left.\varphi=0^{\circ}, \mathrm{q}=30 \mathrm{kN} / \mathrm{m}^{2} ; \mathrm{C}=20 \mathrm{kN} / \mathrm{m}^{2}\right]$
$\sigma_{z}=-2 \times C+q$
$\sigma_{z}=2 \times 20+30$
$\sigma_{z}=-40+30$
$\sigma_{z}=-10 \mathrm{kN} / \mathrm{m}^{2}$
$z_{0}, \sigma_{z}=0$
$18 z_{0}-40-12 z_{0}+30=0$
$6 z_{0}-10=0$

At depth H ,

$$
\mathrm{z}_{0}=1.66 \mathrm{~m}
$$

$$
\begin{aligned}
& \sigma_{\mathrm{H}}=\mathrm{k}_{\mathrm{a}} \gamma \mathrm{z}-2 \mathrm{C} \sqrt{\mathrm{k}_{\mathrm{a}}}-12 \mathrm{H}+\mathrm{k}_{\mathrm{q}} \\
& \sigma_{\mathrm{H}}=18 \mathrm{H}-40-12 \mathrm{H}+30 \\
& \sigma_{\mathrm{H}}=6 \mathrm{H}-10
\end{aligned}
$$

For unsupported depth of excavation, total active thrust must be zero. So,

$$
\begin{aligned}
& \frac{1}{2} \times 10 \times 1.66=\frac{1}{2} \times(H-1.66) \times(6 \mathrm{H}-10) \\
& H=\frac{10}{3}=3.33 \mathrm{~m}
\end{aligned}
$$

59. A 12-hour storm occurs over a catchment and results in a direct runoff depth of 100 mm . The timedistribution of the rainfall intensity is shown in the figure (not to scale). The $\varphi$-index of the storm is (in mm , rounded off to two decimal places) $\qquad$ .

[NAT - 2 Marks]
Ans. 3.60
Sol.

$$
\begin{aligned}
& P=\frac{1}{2}(12+2) \times 20=140 \mathrm{~mm} \\
& \text { w-index }=\frac{P-R}{t}=\frac{140-100}{12}=3.33 \mathrm{~mm} / \mathrm{hr}
\end{aligned}
$$

$$
\begin{aligned}
& \frac{20}{4}=\frac{3.33}{t_{1}} \\
& t_{1}=0.67 \\
& \frac{20}{6}=\frac{3.33}{t_{2}} \\
& t_{2}=0.99 \\
& \varphi=\frac{P_{e}-R}{t_{e}} \\
& =\frac{\left[140-\left[\frac{1}{2} \times 3.33 \times 0.67+\frac{1}{2} \times 3.33 \times 0.99\right]\right]-100}{12-0.67-0.99}=3.60 \mathrm{~mm} / \mathrm{hr}
\end{aligned}
$$

60. A hydraulic jump occurs in a 1.0 m wide horizontal, frictionless, rectangular channel, with a pre-jump depth of 0.2 m and a post-jump depth of 1.0 m . The value of g may be taken as $10 \mathrm{~m} / \mathrm{s}^{2}$. The values of the specific force at the pre-jump and post-jump sections are same and are equal to (in $\mathrm{m}^{3}$, rounded off to two decimal places) $\qquad$
[NAT - 2 Marks]
Ans. 0.60 to 0.64
Sol. Given,
Pre-jump depth
$B=1 \mathrm{~m}=$ width of channel
Post-jump depth
$=y_{1}=0.2 \mathrm{~m}$
As we know,

$$
=y_{2}=1.0 \mathrm{~m}
$$

$$
\begin{array}{ll} 
& \text { Specific force }=F_{S}=\frac{F_{1}+M_{1}}{\rho g} \\
& =A_{1} \cdot z_{1}+\frac{Q^{2}}{g \cdot A_{1}}=A_{2} \cdot z_{2}+\frac{Q^{2}}{g \cdot A_{2}} \\
& \text { Now, } y_{c}^{3}=\frac{y_{1} \cdot y_{2}\left(y_{1}+y_{2}\right)}{2} \\
& =\frac{(0.2 \times 1)(0.2+1)}{2} \\
& \Rightarrow y_{c}^{3}=0.12 \\
& \frac{q^{2}}{g}=0.12 \\
& q^{2}=0.12 \times 10=1.2 \\
Q^{2}=q^{2} \times B^{2}=(1.2) \times(1)^{2}=1.2 \\
& F_{S}=B \cdot y_{1} \cdot \frac{y_{1}}{2}+\frac{Q^{2}}{g \cdot B \cdot y_{1}} \\
& =1 \times 0.2 \times \frac{0.2}{2}+\frac{1.2}{10 \times 1 \times 0.2} \\
& =0.62
\end{array}
$$

61. In Horton's equation fitted to the infiltration data for a soil, the initial infiltration capacity is $10 \mathrm{~mm} / \mathrm{h}$; final infiltration capacity is $5 \mathrm{~mm} / \mathrm{h}$; and the exponential decay constant is $0.5 / \mathrm{h}$. Assuming that the infiltration takes place at capacity rates, the total infiltration depth (in mm ) from a uniform storm of duration 12 h is $\qquad$ (round off to one decimal place)
[NAT - 2 Marks]
Ans. 70.0
Sol. We have given.


[^0]\[

$$
\begin{aligned}
& =\int_{0}^{12} 5+(10-5) \mathrm{e}^{-0.5 \mathrm{t}} \mathrm{dt} \\
& =\left|5 \mathrm{t}+\frac{5 \mathrm{e}^{-0.5 \mathrm{t}}}{-0.5}\right|_{0}^{12} \\
& =\left(5 \times 12-10 \mathrm{e}^{-0.5 \times 12}\right)-\left(5 \times 0-10 \mathrm{e}^{-0.5^{*} 0}\right) \\
& =59.97+10=69.97 \mathrm{~mm} \\
& \simeq 70.0 \mathrm{~mm}
\end{aligned}
$$
\]

62. The composition and energy content of a representative solid waste sample are given in the table. If the moisture content of the waste is $26 \%$, the energy content of the solid waste on dry-weight basis is $\mathrm{MJ} / \mathrm{kg}$ (round off to one decimal place).

| Component | Percent by mass | Energy content as-discarded basis (MJ/kg) |
| :---: | :---: | :---: |
| Food waste | 20 | 4.5 |
| Paper | 45 | 16.0 |
| Carboard | 5 | 14.0 |
| Plastics | 10 | 32.0 |
| Others | 20 | 8.0 |

[NAT - 2 Marks]
Ans. 18.4
Sol.

| Component | Percent by mass | Energy content asdiscarded basis (MJ/kg) | Energy |
| :---: | :---: | :---: | :---: |
| Food waste | 20 | 4.5 | $20 \times 4.5=90$ |
| Paper | 45 | 16.0 | $45 \times 16.0=720$ |
| Cardboard | 5 | 14.0 | $5 \times 14.0=70$ |
| Plastics | 10 | 32.0 | $10 \times 32.0=320$ |
| Others | 20 | 8.0 | $20 \times 8.0=160$ |
|  |  |  | 1360 MJ |

Assume 100 kg of solid waste
Moisture content $=26 \%$
Energy content on dry basis $\quad=\frac{1360}{(100-26)}=18.38 \mathrm{MJ} / \mathrm{kg}$
63. A flocculator tank has a volume of $2800 \mathrm{~m}^{3}$. The temperature of water in the tank is $15^{\circ} \mathrm{C}$, and the average velocity gradient maintained in the tank is $100 / \mathrm{s}$. The temperature of water is reduced to $5 \square \mathrm{C}$, but all other operating conditions including the power input are maintained as the same. The decrease in the average velocity gradient (in \%) due to the reduction in water temperature is (round off to nearest integer).
[Consider dynamic viscosity of water at $15^{\circ} \mathrm{C}$ and $5^{\circ} \mathrm{C}$ as $1.139 \times 10^{-3} \mathrm{~N}-\mathrm{s} / \mathrm{m}^{2}$ and $1.518 \times 10^{-3} \mathrm{~N}-\mathrm{s} / \mathrm{m}^{2}$, respectively]
[NAT - 2 Marks]
Ans. 13
Sol. Case I:

$$
\begin{aligned}
& \mathrm{T}_{1}=15^{\circ} \mathrm{C} \\
& \mathrm{P}_{1}=\mathrm{G}_{1}^{2} \mu_{1} \mathrm{~V} \\
& \mathrm{P}_{1}=\mathrm{G}_{1}^{2} \times 1.139 \times 10^{-3} \times \mathrm{V}
\end{aligned}
$$

## Case II:

$$
\begin{aligned}
& T_{2}=5^{\circ} \mathrm{C} \\
& P_{2}=G_{2}^{2} \mu_{2} V \\
& P_{2}=G_{2}^{2} \times 1.518 \times 10^{-3} \times V
\end{aligned}
$$

Now, Power is the same in both the cases

$$
\begin{aligned}
& \mathrm{P}_{1}=\mathrm{P}_{2} \\
& \mathrm{G}_{1}^{2} \times 1.139 \times 10^{-3} \times \mathrm{V}=\mathrm{G}_{2}^{2} \times 1.518 \times 10^{-3} \times \mathrm{B} \\
& \left(\frac{\mathrm{G}_{1}}{\mathrm{G}_{2}}\right)^{2}=\frac{1.518 \times 10^{-3}}{1.139 \times 10^{-3}} \\
& \left(\frac{\mathrm{G}_{1}}{\mathrm{G}_{2}}\right)^{2}=1.3327 \\
& \frac{\mathrm{G}_{1}}{\mathrm{G}_{2}}=1.154 \\
& \mathrm{G}_{2}=\frac{\mathrm{G}_{1}}{1.154}=0.8665 \mathrm{G}_{1}
\end{aligned}
$$

$\therefore$ \% decrease

$$
\begin{aligned}
& =\frac{G_{1}-G_{2}}{G_{1}} \times 100 \% \\
& =\frac{G_{1}-0.8665 G_{1}}{G_{1}} \times 100 \% \\
& =13.34 \%=13 \%
\end{aligned}
$$

64. The wastewater inflow to an activated sludge plant is $0.5 \mathrm{~m}^{3} / \mathrm{s}$, and the plant is to be operated with a food to microorganism ratio of $0.2 \mathrm{mg} / \mathrm{mg}$-d. The concentration of influent biodegradable organic matter of the wastewater to the plant (after primary settling) is $150 \mathrm{mg} / \mathrm{L}$, and the mixed liquor volatile suspended solids concentration to be maintained in the plant is $2000 \mathrm{mg} / \mathrm{L}$. Assuming that complete removal of biodegradable organic matter in the tank, the volume of aeration tank (in $\mathrm{m}^{3}$, in integer) required for the plant is $\qquad$ .
[NAT - 2 Marks]
Ans. 16200
Sol. Given,

$$
\begin{aligned}
& \mathrm{Q}=0.5 \mathrm{~m}^{3} / \mathrm{sec} \\
& \mathrm{~F} / \mathrm{M}=0.2 \mathrm{mg} / \mathrm{mg} \text {-day } \\
& \mathrm{S}_{0}=150 \mathrm{mg} / \mathrm{L} \\
& X=2000 \mathrm{mg} / \mathrm{L} \\
& \mathrm{~V}=? \\
& \frac{F}{M}=\frac{\mathrm{QS}_{0}}{\mathrm{Vx}} \\
& \frac{0.20 \mathrm{mg}}{\mathrm{mg}-\mathrm{d}}=\frac{\frac{0.5 \mathrm{~m}^{3}}{\mathrm{~S}} \times 86400 \times \frac{150 \mathrm{mg}}{\mathrm{~V}}}{\mathrm{~V}\left(\mathrm{~m}^{3}\right) \times 2000 \mathrm{mg} / \mathrm{l}} \\
& \mathrm{~V}=16200 \mathrm{~m}^{3}
\end{aligned}
$$

65. Trigonometric levelling was carried out from two stations $P$ and $Q$ to find the reduced level (R. L.) of the top of hillock, as shown in the table. The distance between Stations $P$ and $Q$ is 55 m . Assume Stations $P$ and Q , and the hillock are in the same vertical plane. The R. L. of the top of the hillock (in $m$ ) is $\qquad$ (round off to three decimal places).

| Station | Vertical angle of the top of hillock | Staff reading on benchmark | R. L. of benchmrk |
| :---: | :---: | :---: | :---: |
| P | $18^{\circ} 45^{\prime}$ | 2.340 m | 100.000 m |
| Q | $12^{\circ} 45^{\prime}$ | 1.660 m |  |

[NAT - 2 Marks]
Ans. 137.627


We have,

$$
\tan 12^{\circ} 45^{\prime}=\frac{V+0.68}{D+55}
$$

And,
$\tan 18^{\circ} 45^{\prime}=\frac{\mathrm{V}}{\mathrm{D}}$
On solving (i) and (ii), we get

$$
\mathrm{V}=35.287 \mathrm{~m}
$$

$D=103.954 \mathrm{~m}$
Now,
R.L of hill top $=$ R.L of $B M+2.34+V$
$=100+2.34+35.28=137.627 \mathrm{~m}$

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[^0]:    $\therefore$ Total infiltration depth

