## GATE 2020 Civil Engineering

Mega Mock Challenge (02 Jan-03 Jan 2020)

## Questions \& Solutions

1. Direction: In the given question, four words are given of which two are most nearly the same or opposite in meaning. Find the two words and indicate your answer by marking the option which represents the correct combination.
A) Diligent
B) Adorable
C) Meticulous
D) Prominent
A. B-D
B. $\mathrm{A}-\mathrm{C}$
C. $A-B$
D. $A-D$

Ans. B
Sol. The meanings of the words are:
Diligent: having or showing care and conscientiousness in one's work or duties.
Adorable: inspiring great affection or delight.
Meticulous: showing great attention to detail; very careful and precise. Prominent: important; famous.
Hence, option B is the correct answer.
2. Direction: A statement with one blank is given below. Choose the set of words from the given options which can be used to fill the given blank. Despite almost ubiquitous scepticism, the electoral bonds have prevailed and, that too, almost solely $\qquad$ rhetorical claims of "transparency of political funding system," "clean money," and "donor's anonymity." i. with the backing of the ruling government's
ii. based on the endorsement derived from the political party at power's iii. backed by the political party at power's
A. Only i
B. Only ii
C. Only iii
D. Both i and ii

Ans. D
Sol. The given sentence talks about the prevailing nature of 'electoral bonds' in spite of concerns and doubts regarding the same. The sentence goes on to explain that this is occurring because of rhetorical claims
by someone. From the options it is clear that the ruling part is responsible for these 'rhetorical claims'.

Option i - 'backing' means help or support and has been used in conjunction with the correct tense format of the sentence.
Option ii - 'endorsement' also means help or support and it tallies with the sentence structure.
Option iii - although 'backed' has been used it is in the incorrect tense form. This makes it incorrect.
Thus, option D is the correct answer.
3. Which letter-cluster will replace the question mark (?) in the following series?
HQCF, MVHK, JSEH, OXLM, ?
A. FTRD
B. LUGJ
C. MKOP
D. SWQ

Ans. B
Sol. Pattern is-


Hence, the correct answer is option B.
4. Three different positions of the same dice are shown. Which symbol will be on the face opposite to the one having '*'?

A. +
B. !
C. $\$$
D. @

Ans. A
Sol. Pick out the dices in which one symbol is common, after that arrange them in ACW or CW direction.

In II and III ' + ' is common

+ = @
*     + ! \$

Interchange the missing symbol '*' with repeated symbol '+' Hence, option (A) is the correct answer.
5. In the following diagram, the triangle represents 'Dentists', the circle represents 'Professors' and the rectangle represents 'Doctors'. The numbers in different segments show the number of persons.


How many professors are dentists but not doctors?
A. 17
B. 9
C. 15
D. 13

Ans. B
Sol. Given diagram is-

circle represents Professors rectangle represents Doctors triangle represents Dentists No. of professors who are dentists but not doctors=2+7=9
Hence, the correct answer is option B.
6. In the following question, some statements followed by some conclusions are given. Taking the given statements to be true even if they seem to be at variance from commonly known facts, read all the
conclusions and then decide which of the given conclusions logically follows the given statements.

## Statement:

Parents must understand that their child cannot attain excellence on his own. He needs their support. They must thus be open to help him at various steps rather than merely setting high expectations.

## Conclusion:

I. Ideal students are not born ideal or perfect. They are nurtured to become ideal by their educators. The environment at home has a great impact on the way a student performs in school.
II. The life of an ideal student may seem tough from a distance. However, it is actually much more sorted as compared to those who procrastinate and do not give complete attention to their studies.
A. If only conclusion I follows
B. If only conclusion II follows
C. If both I and II conclusion follow
D. If neither I nor II conclusion follows

Ans. A
Sol. Conclusion I follows, based on the given statement a major component in the making of an Ideal student is described that it takes efforts not only from the students but also from the educators( Teachers and Parents)
Conclusion II is a correct statement that is the hard work and struggle that it takes to become an ideal student but it cannot be the conclusion of the given statement.
7. Direction: Each question below is followed by two statements I and II. You have to determine whether the data given in the statement is sufficient for answering the question. You should use the data and your knowledge of Mathematics to choose the best possible answer.

A man deposited Rs. ' $x$ ' in bank which gives simple interest at the rate of $8 \%$ $p . a$. Find the value of ' $x$ '.
Statement I: After 3 years, amount received by him is Rs. $(x+672)$.
Statement II: Interest earned by him after 3 years is $24 \%$ of the amount deposited by him.
A. If the data in Statement I alone are sufficient to answer the question, while the data in Statement II alone are not sufficient to answer the question.
B. If the data in Statement II alone are sufficient to answer the question, while the data in Statement I alone are not sufficient to answer the question.
C. If the data either in Statement I or in Statement II alone are sufficient to answer the question.
D. If the data in both Statements I and II together are necessary to answer the question.
Ans. A
Sol. Statement I:
Simple interest earned by him
$=x+672-x=$ Rs. 672
So, $672=\frac{x \times 8 \times 3}{100}$
$x=$ Rs. 2800
So, statement I alone is sufficient to answer the question.
Statement II:
We have to calculate principal(x) but we are not given interest since it is also in form of $x$. Hence, there are 2 unknowns.
Statement II alone is not sufficient to answer the question.
Thus, the data in Statement I alone are sufficient to answer the question, while the data in Statement II alone are not sufficient to answer the question.
So option (A) is the correct answer.
8. The given pie chart shows the breakup of total number of the
employees of a company working in different offices (A, B, C, D and E). Total no. of employees $=2400$


What is the number of offices in which the number of employees of the company is between 350 and 650?
A. 3
B. 4
C. 2
D. 1

Ans. A
Sol. Total no. of Employees $\left(360^{\circ}\right)=$ 2400
No. of employees in office $A\left(126^{\circ}\right)$
$=\frac{2400}{360} \times 126=840$
No. of employees in office $B\left(18^{\circ}\right)$
$=\frac{2400}{360} \times 18=120$
No. of employees in office $C\left(54^{\circ}\right)$
$=\frac{2400}{360} \times 54=360$
No. of employees in office $D\left(90^{\circ}\right)$
$=\frac{2400}{360} \times 90=600$
No. of employees in office $E\left(72^{\circ}\right)$
$=\frac{2400}{360} \times 72=480$
Number of offices in which the number of employees of the company is between 350 and $650=3$
9. Find the numbers $a, b, c$ between 2 and 18 such that
I. their sum is 25 ,
II. the numbers $2, a, b$ are
consecutive terms of an A.P. and
III. The numbers b, c, 18 are
consecutive terms of a G.P.
A. $a=5, b=8, c=12$
B. $a=7, b=8, c=12$
C. $a=5, b=9, c=11$
D. $a=7, b=5, c=11$

Ans. A

Sol. We have a + b + c = 25
$2, a, b$ are in A.P. $\Rightarrow 2 a=2+b$
b, c, 18 are in G.P. $\Rightarrow 18 \mathrm{~b}=\mathrm{c} 2$
Substituting for $a$ and $b$ in (1), using relations (2) and (3), we get
$\Rightarrow 1+\frac{b}{2}+\frac{c^{2}}{18}+c=25$
$\Rightarrow c^{2}+12 \mathrm{c}-288=0$
$\Rightarrow(c-12)(c+24)=0$
$\Rightarrow C=12$ or $\mathrm{c}=-24$
Since the numbers lie between 2 \&
18,
We take $\mathrm{c}=12$
$\Rightarrow \mathrm{a}+\mathrm{b}=13$
$\Rightarrow a+2 a-2=13$
$\Rightarrow b=8, a=5$
10. Statements:

All lions are ducks.
No duck is a horse.
All horses are fruits.

## Conclusions:

I. No lion is a horse.
II. Some fruits are horses.
III. Some ducks are lions.
IV. Some lions are horses.
A. Only either I or II and III \& IV follow
B. Only either I or IV and both II and III follow
C. Only either I or IV and II follow
D. Only Conclusion I \& II and III follow
Ans. D
Sol.


We use elimination to find an exception to the generality of the question. Thus we prove they are not implied. The diagram above satisfy all the above statement but contradict with the conclusion (iv). Since we found an exception, the conclusion is not true in every case. Thus it is not implied.

We can draw many scenarios that satisfy the statements using Venn diagram \& check for the validity of the conclusions.
Conclusions (i), (ii), (iii) hold good for every case so they are implied.
11. All queens and kings are removed from a deck of playing cards. Ace will be considered as 1 and jack will be considered as 0 . You took out 4 cards. The probability that all cards will be in order (order is -
1234,0123,2345,6789....) from the same deck is M . What is ( $\mathrm{M} \times 10^{5}$ )?
A. 0.982
B. 0.700
C. 0.643
D. 0.500

Ans. A
Sol. Total cards $=52-4-4=44$
(cards)
$\rightarrow$ You took out 4 cards
$\therefore$ (1) Let order be 0, 1, 2, 3
So, $\quad P($ order $-0,1,2,3)=\frac{4}{44} \times \frac{1}{43} \times \frac{1}{42} \times \frac{1}{41}=x$
$\therefore$ Lost possible set $-(7,8,9,10)-8$
such sets are possible
$\therefore \mathrm{P}($ Total $)=8 \times \mathrm{P}$ (order $-0,1,2$,
3) $=8 x$
$\Rightarrow 8 \mathrm{x} \times 10^{5}=0.982$
12. The divergence of the vector field
$\vec{V}=\left(x^{2}+y\right) \widehat{i}+(z-2 x y) \widehat{j}+(x y) \widehat{k}$ at
$(1,1,1)$ is
A. 1
B. -1
C. 0
D. 2

Ans. C
Sol.
$\nabla \cdot \vec{V}=\frac{\partial}{\partial x}\left(x^{2}+y\right)+\frac{\partial}{\partial y}(z-2 x y)+\frac{\partial}{\partial z}(x y)$
$=2 x-2 y$
$=0$ at $(1,1,1)$
13. Using trapezoidal rule for the table given below

| $x:$ | 4 | 4.2 | 4.4 | 4.6 | 4.8 | 5.0 | 5.2 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\operatorname{Ln} x:$ | 1.39 | 1.44 | 1.48 | 1.53 | 1.57 | 1.61 | 1.65 |

Find the value of Integral
$I=\int_{4}^{5.2} \ln x . d x$
A. 1.83
B. 1.93
C. 1.64
D. 0.98

Ans. A
Sol. Width (h) =0.2
$\int_{4}^{5.2} \ln x d x=\frac{h}{2}\left[\left(y_{0}+y_{6}\right)+2\left(y_{1}+y_{2}+y_{3}+y_{4}+y_{5}\right)\right]$
$\begin{aligned} \int_{4}^{5.2} \ln x d x & =\frac{0.2}{2}[(1.39+1.65)+2(1.44+1.48+1.53+1.57+1.61)] \\ & =1.83\end{aligned}$
14. Which of the following statements is incorrect?
A. Light weight concrete is a special type of concrete having density $2300 \mathrm{~kg} / \mathrm{m} 3$
B. High density concrete is used for atomic power plants.
C. Slump Value of self consolidated concrete lies between 650-750.
D. Fatty acids or alcohols in the concrete produce air entrained concrete.
Ans. A
Sol. Light weight concrete has density lesser than $1930 \mathrm{~kg} / \mathrm{m} 3$. Other statements are correct.
15. Which of the following is the most appropriate theory for ductile material among following?
A. Maximum distortion energy theory
B. Max strain theory
C. Max shear stress theory
D. Maximum strain energy theory

Ans. A
Sol. The most appropriate theory for ductile material is 'maximum distortion energy theory' as this theory is in perfect agreement with the case of pure shear. According to this theory, maximum shear strain energy in a body should be less than or equal to maximum shear strain energy under uniaxial loading.
16. What is the anchorage length of a tie bar, which is bent through $135^{\circ}$ round a bar of diameter 12 mm ?
A. 72 mm
B. 96 mm
C. 48 mm
D. 192 mm

Ans. A

Sol. For secondary reinforcement such as stirrups in beams and transverse ties in a column, complete development length and anchorage shall be deemed to have been provided when the bar is bent through an angle of $135^{\circ}$ and is continued beyond the end of the curve for a length of at least 6 diameters.
So, $6 \varphi=6 \times 12=72 \mathrm{~mm}$
17. A sample of soil weighs 153 grams. Its clay fraction weighs $34 \%$ of the total weight. If it's liquid limit is $62 \%$ and the Plastic limit is $27 \%$. Classify the soil.
A. MH
B. CL
C. CH
D. Organic Clay

Ans. C
Sol. Plasticity Index, IP = LL- PL = 62 27 = 35\%
$\mathrm{I}_{\mathrm{p}}$ of A-line
$\mathrm{I}_{\mathrm{p}}=0.73(\mathrm{LL}-20)=30.66$
Therefore Clay
Also LL>50.
Therefore highly compressible.
18. A canal and a river runs parallel at an average of 76 m apart. The elevation of water level in river is at +311.2 m and in the canal is at +320.04 m . A stratum of sand intersects both the river and canal below the water level. The sand layer is 1.80 m thick, and is sandwiched between layers of impervious clay with the area as $24 \mathrm{~m}^{2}$. The seepage loss in the canal is if the permeability of sand is $2 \times 10^{-}$ ${ }^{3} \mathrm{ft} / \mathrm{sec}$.
A. $1.7 \times 10^{-3} \mathrm{~m}^{3} / \mathrm{s}$
B. $2.7 \times 10^{-3} \mathrm{~m}^{3} / \mathrm{s}$
C. $1.6 \times 10^{-3} \mathrm{~m}^{3} / \mathrm{s}$
D. $2.8 \times 10^{-3} \mathrm{~m}^{3} / \mathrm{s}$

Ans. A
Sol. $\mathrm{q}=\mathrm{k} \frac{\Delta h}{L} \mathrm{~A}$
$\mathrm{k}=2 \times 10^{-3} \times 0.3048=0.61 \times 10^{-}$
${ }^{3} \mathrm{~m} / \mathrm{sec}$
$\mathrm{q}=\frac{0.61 \times 10^{-3} \times(320.04-311.2) \times 24}{76}=$
$1.7 \times 10^{-3} \mathrm{~m}^{3} / \mathrm{s}$
19. Match List I and List II and select the correct answer using the codes given below the lists.

| List I | List II |
| :--- | :--- |
| P. Reservoir Routing | 1. Uses equation of continuity <br> and St.Venant's equation |
| Q. Channel Routing | 2. Uses equation of continuity |
| R. Hydraulic routing | 3. Unique function of both <br> flood inflow and outflow |
| S. Hydrologic routing | 4. Uses Modified puls method |

A. P-4 Q-1 R-3 S-2
B. P-3 Q-4 R-1 S-3
C. P-4 Q-3 R-1 S-2
D. P-3 Q-1 R-2 S-4

Ans. C
Sol. The equation of continuity used in all hydrologic routing as the primary equation states that the difference between the inflow and outflow rate is equal to the rate of change of storage, i.e.
$I-Q=\frac{d S}{d t}$
I - inflow; Q - outflow; S - storage The modified puls routing method is probably most often applied to reservoir routing considering the finite difference form of the continuity equation.
Hydraulic routing combines the continuity equation with a very simplified form of the St. Venant equations.
In Channel routing, the storage is a function of both inflow and outflow.
20. A weir consists of a 36 m long horizontal floor with two sheet piles of 6 m and 8 m depth at the upstream and downstream end of the floor, respectively. Under an impounded depth of 4 m , the residual head at centre is $\qquad$ m.
A. 2.12
B. 3.15
C. 5.56
D. 1.1

Ans. A
Sol. Creep length, $L=36+(2 \times 6)+(2$ $\times 8)$
$=64 \mathrm{~m}$


At the mid-point of floor, creep length
$=(6+6+18)=30 \mathrm{~m}$
Loss in seepage head at centre,
$H_{L}=\frac{4 \times 30}{64}=1.875 \mathrm{~m}$
Residual seepage head at centre $=\mathrm{H}$
$-H_{L}=4.0-1.875=2.125 m$
21. Match List $\mathbf{I}$ with List-II and select the correct answer.

|  | List-I (Control Structures) |  | List-II (Functions) |
| :--- | :--- | :--- | :--- |
| P | Canal Drop | 1. | Control of flow Depth |
| Q | Canal Escape | 2. | Control of Bed Grade |
| R | Canal Cross Regulation | 3. | Control of full supply level |
| S | Canal Outlet | 4. | Control of discharge |

A. $\mathrm{P}-2 \mathrm{Q}-3 \mathrm{R}-4 \mathrm{~S}-1$
B. P-3 Q-2 R-1 S-4
C. P-3 Q-2 R-4 S-1
D. P-2 Q-3 R-1 S-4

Ans. D
Sol.

| Canal Drop - Control of Bed Grade |
| :--- |
| Canal Escape - Control of full supply level |
| Canal Cross Regulation - Control of flow depth |
| Canal Outlet - Control of disharge |

22. In a tidal model, the horizontal scale ratio is $\frac{1}{750}$ and the vertical scale is $\frac{1}{75}$.The model period (in minutes), corresponding to a prototype period of 18 hours, would be
A. 11.24
B. 12.47
C. 14.96
D. None of these

Ans. B

Sol. $L_{r}=\frac{1}{750}$ and $h_{r}=\frac{1}{75}$
From Froude's law, $\frac{V_{r}}{\sqrt{h_{r}}}=1$
$\Rightarrow$ Timeratio, $T_{r}=\frac{L_{r}}{\sqrt{h_{r}}}=\frac{1 / 750}{\sqrt{1 / 75}}$
$=0.01155\left(\because V_{r} \frac{L_{r}}{T_{r}}\right)$
$\Rightarrow \quad$ Model period, $T_{m}=T_{p} \times T_{r}$
$=(18 \times 60 \times 60) \times 0.01155=748.24 \mathrm{~s}$
$\therefore T_{m}=12.47 \mathrm{~min}$ utes
23. For a steady incompressible laminar flow through a circular pipe, the velocity distribution is.
A. Linear with zero value at the surface and maximum value at the centre.
B. Linear with zero value at the centre and maximum value at the surface.
C. Parabolic with zero value at the surface and maximum value at the centre
D. Parabolic with zero value at the centre and maximum value at the surface.

Ans. C
Sol.


Velocity
variation $V=\frac{1}{4 \mu}\left[\frac{-\partial P}{\partial x}\right]\left(R^{2}-r^{2}\right)$
24. Elevation and temperature for places
$\mathrm{A}, \mathrm{B}$ and C are tabulated below :

| Place | Elevation $(\mathrm{m})$ | Temperature $\left({ }^{\circ} \mathrm{C}\right)$ |
| :---: | :---: | :---: |
| A | 45 | 21 |
| B | 350 | 18.5 |
| C | 500 | 14 |

Based on above data, for dry air lapse rate of place $A$ to $B$ and place $B$ to $C$ respectively are
A. Sub-adiabatic and Sub-adiabatic
B. Super-adiabatic and Sub-adiabatic
C. Sub-adiabatic and Super-adiabatic
D. Super-adiabatic and Superadiabatic

Ans. C
Sol. From place A to B, Ambient lapse rate $=$
$\frac{21-18.5}{350-45} \times 1000=8.196<9.8^{\circ} \mathrm{C} / \mathrm{km}$
Hence the condition is sub-adiabatic From place B to C, Ambient Lapse rate =
$\frac{18.5-14}{500-350} \times 1000=30{ }^{\circ} \mathrm{C} / \mathrm{km}>9.8^{\circ} \mathrm{C} / \mathrm{km}$
So, the condition is super-adiabatic.
25. At a certain location, the cumulative noise power distribution curve is given below:


The value of $L_{50}$ (in $d B$ ) is equal to
A. 95
B. 90
C. 85
D. 80

Ans. A
Sol. $\mathrm{L}_{50}$ is the sound pressure level in dB which is exceeded for $50 \%$ of the gauging time.
Now, slope of the given curve
$=\frac{100-0}{120-70}=\frac{100}{50}=2$
$\because$ Curve is a straight line, hence slope is constant.
$\Rightarrow 2=\frac{50-0}{L_{50}-70}$
$\Rightarrow L_{30}-70=25$
$\Rightarrow L_{50}=95 \mathrm{~dB}$
26. A line of levels was run from a benchmark of RL 51.540 and ended on a benchmark of RL 63.200. The sum of backsights and foresights were 86.755 m and 72.725 m respectively. The closing error of the work was $\qquad$ m.
A. 2.86
B. 2.37
C. 2.46
D. 2.29

Ans. B
Sol.

$$
\begin{aligned}
\text { Measured value } & =\Sigma \mathrm{B} . \mathrm{S}-\Sigma \mathrm{F} . \mathrm{S} \\
& =86.755-72.725 \\
& =14.03 \mathrm{~m} \\
\text { True value } & =\text { Last } \mathrm{R} . \mathrm{L}-\text { First } \mathrm{R} . \mathrm{L} \\
& =63.2-51.54 \\
& =11.66 \mathrm{~m}
\end{aligned}
$$

Error $=$ Measured value - true value

$$
=14.03-11.66=2.37 \mathrm{~m}
$$

27. A road section of length 2 km scale 16 cm on a vertical photograph. The focal length of the camera is 320 mm . If the terrain is fairly level, then the flying height will be $\qquad$ km.
A. 2.0
B. 4.0
C. 1.6
D. 2.4

Ans. B
Sol.

$$
\text { Scale }=\frac{f}{H}
$$

$$
\frac{16}{2000 \times 100}=\frac{32}{H}
$$

$H=400000 \mathrm{~mm}=4 \mathrm{~km}$
28. A steel reel of mass $m$ \& radius $r$ \& radius of gyration $k$ is rolling down from rest. From its one end, a thread wounded on it is held on the ceiling as shown. Find linear acceleration on the reel.

A. $\frac{\mathrm{gr}}{\mathrm{r}^{2}+\mathrm{k}^{2}}$
B. $\frac{\mathrm{gr}^{2}}{\mathrm{r}^{2}+\mathrm{k}^{2}}$
C. $\frac{\mathrm{gr}^{r}}{\mathrm{r}^{2}+\mathrm{k}}$
D. $\frac{\mathrm{gr}}{\mathrm{r}+\mathrm{k}^{2}}$

Ans. B
Sol. Area moment of inertia $=\mathrm{I}=\mathrm{mr}^{2}+\mathrm{mk}^{2}$ Generated torque about point $\mathrm{O}=\mathrm{T}=\mathrm{Ia}=\mathrm{ma}\left(\mathrm{r}^{2}+\mathrm{k}^{2}\right)$,
Moment about point $\mathrm{O}=\mathrm{M}=\mathrm{T}=\mathrm{mgr}$.
So, $m g r=m a\left(r^{2}+k^{2}\right)$
Or, $a=\frac{\mathrm{gr}^{2}}{\mathrm{r}^{2}+\mathrm{k}^{2}}$
$a=a r=\frac{\mathrm{gr}^{2}}{\mathrm{r}^{2}+\mathrm{k}^{2}}$
29. A steel wire of $G=85 \mathrm{GPa}$ is to be proportioned such that the maximum shearing stress is 80 MPa for an angle of twist of $90^{\circ}$. The length to diameter ratio is $\qquad$
Sol. Angle of twist $=90^{\circ}=90 \times \frac{\pi}{180} \mathrm{rad}$
$\frac{\tau_{\max }}{\mathrm{R}}=\frac{\mathrm{G} \theta}{\mathrm{L}}$
$\frac{\tau_{\max }}{d / 2}=\frac{G \theta}{L}$
$\frac{\mathrm{L}}{\mathrm{d}}=\frac{\mathrm{G} \theta}{2 \tau_{\max }}$
$\frac{\mathrm{L}}{\mathrm{d}}=\frac{85 \times 10^{3} \times 90 \times \frac{\pi}{180}}{2 \times 80}$
$\frac{\mathrm{L}}{\mathrm{d}}=834.48$
30. A 10m high retaining wall retains dry sand. Initially the soil is in loose state with void ratio of $0.5, Y_{d}=17.4$ $\mathrm{kN} / \mathrm{m}^{3}$ and $\varphi=30^{\circ}$. Subsequently, the soil is compacted and filled to the same height and now its new void ratio is $0.4, Y_{d}=18.4 \mathrm{kN} / \mathrm{m}^{3}$ and $\varphi=$ $35^{\circ}$. According to Rankine's earth pressure theory, the ratio of initial passive pressure to final passive pressure is $\qquad$ _.

Sol. Initial passive pressure (for $\mathrm{c}=0$ soil)
$\gamma=\gamma_{d}=17.4 \mathrm{kN} / \mathrm{m}^{3}, \varphi=30^{\circ}$
$\mathrm{P}_{\mathrm{p} 1}=\mathrm{K}_{\mathrm{p}} \gamma \mathrm{H}$ and $\mathrm{K}_{\mathrm{p}}=\left(\frac{1+\sin \varnothing}{1-\sin \emptyset}\right)=3$
$P_{p 1}=3 \times 17.4 \times 10=522 \mathrm{kN} / \mathrm{m}^{2}$
Final passive pressure
$\gamma=\gamma_{d}=18.4 \mathrm{kN} / \mathrm{m}^{3}, \varphi=35^{\circ}$
$\mathrm{P}_{\mathrm{p} 2}=\mathrm{K}_{\mathrm{p}} \gamma \mathrm{H}$ and $\mathrm{K}_{\mathrm{p}}=\left(\frac{1+\sin \emptyset}{1-\sin \emptyset}\right)=$
3.69
$\mathrm{P}_{\mathrm{p} 2}=3.69 \times 18.4 \times 10=678.96$
$\mathrm{kN} / \mathrm{m}^{2}$
Therefore, $\frac{P_{P 1}}{P_{P 2}}=\frac{522}{678.96}=0.768$
31. A centrifugal pump needs 1000 W of power when operating at 2000 rpm . If the speed of pump is increased to 4000 rpm , then power requirement is
$\qquad$ kW.
Sol. By similarity laws,

$$
\frac{P_{1}}{\gamma D_{1}^{5} N_{1}^{3}}=\frac{P_{2}}{\gamma D_{2}^{5} N_{2}^{3}}
$$

For a centrifugal pump, $D_{1}=D_{2}$
$\therefore \quad \frac{P_{1}}{N_{1}^{3}}=\frac{P_{2}}{N_{2}^{3}}$
$\Rightarrow \quad \frac{1000}{(2000)^{3}}=\frac{P_{2}}{(4000)^{3}}$
$\Rightarrow \quad 2^{3} \times 1000=P_{2}$
$\therefore \quad P_{2}=8000 \mathrm{~W}=8 \mathrm{~kW}$
32. The turning radius for subsonic aircraft having wheel base of 17.50 m , thread of main loading gear is 6.62, turning speed of 45 kmph and coefficient of friction between tyre and pavement equal to 0.13 is __m. (up to 2 decimal places)
Sol. The turning radius is the maximum of:

1. $\mathrm{R}=\frac{V^{2}}{125 f}=\frac{45^{2}}{125 \times 0.13}=124.615 \mathrm{~m}$
2. From Horonjeff's equation
$=\frac{0.388 W^{2}}{\frac{T}{2}-S}$
$\mathrm{S}=6+\frac{6.62}{2}=9.31$
$\mathrm{T}=22.5 \mathrm{~m}$
$R=\frac{0.388 \times 17.50^{2}}{\frac{22.5}{2}-9.31}$
$R=61.25 \mathrm{~m}$
3. Absolute minimum turning radius for subsonic aircraft regardless of speed $=120 \mathrm{~m}$.
Radius of turning $=124.615 \mathrm{~m}$
4. A 3.5 m reinforced concrete slab having thickness 20 cm and $\mathrm{f}=1.5$ with bars of diameter 1.0 cm at 0.3 m spacing.
Given that allowable working stress in steel in tension is $1200 \mathrm{~kg} / \mathrm{cm}^{2}$ and unit weight of concrete is $2400 \mathrm{~kg} / \mathrm{m}^{3}$. The spacing between contraction joints in this case is $\qquad$ m
[rounded to two decimal places]
Sol.

$A_{\text {st }}=$ number of bars $\times \frac{\pi}{4} \mathrm{~d}^{2}$
$\mathrm{A}_{\text {st }}=\frac{3.5}{0.3} \times \frac{\pi}{4} \times 1^{2}=9.16 \mathrm{~cm}^{2}$
$\sigma_{s t} \mathrm{~A}_{\text {st }}=\mathrm{B} \times \frac{\mathrm{L}}{2} \times \mathrm{h} \times \gamma \times \mathrm{f}$
$1200 \times 9.16=3.5 \times \frac{\mathrm{L}}{2} \times \frac{20}{100} \times 2400 \times 1.5$
$\therefore$ spacing between contraction joint
$\mathrm{L}_{\mathrm{C}}=\frac{1200 \times 9.16 \times 200}{3.5 \times 20 \times 2700 \times 1.5}$
$\mathrm{L}_{\mathrm{c}}=8.72 \mathrm{~m}$
5. A speed study is conducted to have a knowledge about the time mean speed and space mean speed. The data collected is as below-

| Speed range $(\mathrm{m} / \mathrm{s})$ | $2-5$ | $6-9$ | $10-13$ | $14-17$ |
| :--- | :--- | :--- | :--- | :--- |
| Frequency $(\mathrm{q})$ | 10 | 14 | 0 | 9 |

The ratio of space mean speed to time mean speed is $\qquad$ [correct to three decimal places]
Sol.

| Speed Range | Average speed | $q_{i}$ | $V_{i} q$ | $\frac{q_{i}}{V_{i}}$ |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $2-5$ | 3.5 | 10 | 35 | 2.857 |  |  |  |  |
| $6-9$ | 7.5 | 14 | 105 | 1.867 |  |  |  |  |
| $10-13$ | 11.5 | 0 | 0 | 0 |  |  |  |  |
| $14-17$ | 15.5 | 9 | 139.5 | 0.581 |  |  |  |  |
| $\sum$ |  |  |  |  |  | $\sum q_{i}=33$ | $\sum \mathrm{~V}_{i}=279.5$ | $\sum q_{i}$ |
|  |  |  | $V_{i}$ |  |  |  |  |  |

Time mean speed

$$
=\frac{\sum \mathrm{V}_{\mathrm{i}} \mathrm{q}_{\mathrm{i}}}{\sum \mathrm{q}_{\mathrm{i}}}=\frac{279.5}{33}=8.47 \mathrm{~m} / \mathrm{sec}
$$

Space mean speed

$$
=\frac{\sum q_{\mathrm{i}}}{\frac{\sum \mathrm{q}_{\mathrm{i}}}{\mathrm{~V}_{\mathrm{i}}}}=\frac{33}{5.305}=6.22 \mathrm{~m} / \mathrm{sec} .
$$

Hence
$\frac{\text { Space mean speed }}{\text { Timemean speed }}=\frac{6.22}{8.47}=0.734$
35. The average and saturation flows on cross roads $A$ and $B$ during design period are as follows-

|  | Average flow | Saturation flow |
| :--- | :--- | :--- |
| Road A | $350 \mathrm{pcu} / \mathrm{hr}$ | $1120 \mathrm{pcu} / \mathrm{hr}$ |
| Road B | $210 \mathrm{pcu} / \mathrm{hr}$ | $900 \mathrm{pcu} / \mathrm{hr}$ |

The total lost time for the signal design to be considered as 16 seconds. The green time for road $B$ is
$\qquad$ seconds.
Sol.
$y_{a}=\frac{q_{a}}{S_{a}}=\frac{350}{1120}=0.3125 \quad y_{b}=\frac{q_{b}}{s_{b}}=\frac{210}{900}=0.2333$
$Y=y_{a}+y_{b}=0.3125+0.2333=0.5458$
$\mathrm{L}=16$ seconds
$C_{0}=\frac{1.5 L+5}{1-Y}=\frac{1.5 \times 16+5}{1-0.5458}=63.85$ seconds
$G_{b}=\frac{Y_{a}}{Y}\left(c_{0}-c\right)=\frac{0.2333}{0.5458}(63.85-16)=20.45$ seconds
36. The solution curve of the differential equation $x \frac{d y}{d x}=y+2 x^{3}$ passes through the point $(1,0)$. Then among the points given below, the curve also passes through:
A. $(-1,0)$
B. $(0,-1)$
C. $(2,10)$
D. $(-2,6)$

Ans. A
Sol. The differential equation can be written as:
$\frac{d y}{d x}=\frac{y}{x}+2 x^{2}$
$\frac{d y}{d x}-\frac{y}{x}=2 x^{2}$
which is a linear equation in ' $y^{\prime}$
$I F=e^{-\int \frac{1}{x} d x}=e^{-\log x}=-\frac{1}{x}$
Thus, the solution of the equation will be:
$y x I F=\int 2 x^{2} x I F d x$
$-\frac{y}{x}=-\int 2 x d x$
$\frac{y}{x}=x^{2}+C$
Putting $(1,0)$ we get $C=-1$
Thus,
$\frac{y}{x}=x^{2}-1$
Clearly only ( $-1,0$ ) satisfies the above equation.
37. Find the Laplace transform of the function $f(t)$ given as
$f(t)=(t-2)^{2}$
A. $\frac{4}{s}-\frac{4}{s^{2}}+\frac{2}{s^{3}}, s>0$
B. $\frac{4}{s}-\frac{4}{s^{2}}+\frac{4}{s^{3}}, s>0$
C. $\frac{4}{s}-\frac{2}{s^{2}}+\frac{2}{s^{3}}, s>0$
D. $\frac{2}{s}-\frac{4}{s^{2}}+\frac{2}{s^{3}}, s>0$

Ans. A
Sol. Given $\mathrm{L}\left[(t-2)^{2}\right]$
$=\lim _{T \rightarrow \infty} \int_{0}^{T}(t-2)^{2} e^{-s t} d t$
Using integration by parts with $u^{\prime}$ $=e^{-s t}$ and $\mathrm{V}=(t-2)^{2}$ we will
find,
$\int_{0}^{T}(t-2)^{2} e^{-s t} d t=-\left[\frac{(t-2)^{2} e^{-s t}}{s}\right]_{0}^{T}+$
$\frac{2}{s} \int_{0}^{T}(t-2) e^{-s t} d t$
$=\frac{4}{s}-\frac{(T-2)^{2} e^{-s T}}{s}+$
$\frac{2}{s} \int_{0}^{T}(t-2) e^{-s t} d t$
thus,
$\lim _{T \rightarrow \infty} \int_{0}^{T}(t-2)^{2} e^{-s t} d t=\frac{4}{s}+\frac{2}{s} \lim _{T \rightarrow \infty} \int_{0}^{T}(t-2) e^{-s t} d t$
Using by parts with $u^{\prime}=e^{-s t}$ and $v$ $=t-2$ we find
$\int_{0}^{T}(t-2) e^{-s t} d t=\left[-\frac{(t-2) e^{-s t}}{s}-\frac{1}{s^{2}} e^{-s t}\right]_{0}^{T}$
Let $\mathrm{T} \rightarrow \infty$ in the above expression we will get
$\operatorname{Lim}_{T \rightarrow \infty} \int_{0}^{T}(t-2) e^{-s t} d t=-\frac{2}{s}+\frac{1}{s^{2}}, s>0$ Hence,
$F(s)=\frac{4}{s}+\frac{2}{s}\left(-\frac{2}{s}+\frac{1}{s^{2}}\right)=\frac{4}{s}-\frac{4}{s^{2}}+$
$\frac{2}{s^{3}}, s>0$
38. The state of stress of an element is given as follows.
The value of normal stress are $\sigma_{x}=30 \mathrm{MPa}$
, $\sigma_{y}=-10 M P a$ and major principal stress $\sigma_{1}=50 \mathrm{MPa}$. Then the value of shear stress is.
A. 30 MPa
B. 25.20 MPa
C. 45.90 MPa
D. 34.64 MPa

Ans. D
Sol. We know
$\sigma_{1}=\frac{\sigma_{x}+\sigma_{y}}{2}+\sqrt{\left(\frac{\sigma_{x}-\sigma_{y}}{2}\right)^{2}+\left(\tau_{x y}\right)^{2}}$
$50=\frac{30+(-10)}{2}+\sqrt{\left(\frac{30-(-10)}{2}\right)^{2}+\left(\tau_{x y}\right)^{2}}$
$\left(\tau_{x y}\right)=\sqrt{40^{2}-20^{2}}$
$\left(\tau_{x y}\right)=34.64 M P a$
39. Determine the vertical reaction at support A

A. $\frac{5 \mathrm{P}}{18}$
B. $\frac{3 \mathrm{P}}{18}$
C. $\frac{13 \mathrm{P}}{18}$
D. $\frac{15 \mathrm{P}}{18}$

Ans. C
Sol.


Displacement at C will be equal for both sections
$\mathrm{D}_{1}=\mathrm{D}_{2}$
$\frac{\mathrm{PL}^{2}}{2 \mathrm{EI}} \times \mathrm{L}+\frac{\mathrm{PL}^{3}}{3 \mathrm{EI}}-\frac{\mathrm{R}(2 \mathrm{~L})^{3}}{3 \mathrm{EI}}=\frac{\mathrm{R} \times(\mathrm{L})^{3}}{3 \mathrm{EI}}$
$\frac{5 \mathrm{PL}^{3}}{6 \mathrm{EI}}=\frac{9 \mathrm{RL}^{3}}{3 \mathrm{EI}}$
$R=5 P / 18$
$R_{A}+R=P$
$R_{A}=13 P / 18$
40. A two hinged semicircular arch of uniform square cross section having side of 80 mm , spanning 20 m , is subjected to rise in temperature of $30^{\circ}$. Find horizontal thrust produced at supports. Take $\mathrm{E}=2 \mathrm{x}$ $10^{5} \mathrm{~N} / \mathrm{mm}^{2}$ and $\mathrm{a}=12 \times 10^{-6}$ per ${ }^{\circ} \mathrm{C}$.
A. 3.129 kN
B. 9.387 kN
C. 37.58 kN
D. 18.29 kN

Ans. A
Sol. For two hinged arch,
$H=(4 E I a T) / \pi R^{2}$
Here, $\mathrm{I}=\mathrm{d}^{4} / 12=(80)^{4} / 12$
$H=\left(4 \times 2 \times 10^{5} \times(80)^{4} \times 12 \times 10^{-}\right.$
$\left.{ }^{6} \times 30\right) /\left(12 \times \pi \times(10000)^{4}\right)$
$\mathrm{H}=3.129 \mathrm{kN}$
41. An isolated simply supported T-beam, constructed using M20 concrete and Fe250 steel, having effective span of 9 m and cross-sectional dimensions as shown in the figure is subjected to a working moment of 270 kNm. If neutral axis lies in the web, then the depth of neutral axis is $\qquad$ mm .

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[The beam is designed using WSM]

A. 157.62
B. 167.62
C. 177.62
D. 187.62

Ans. D
Sol. Effective span, $\mathrm{L}_{0}=9000 \mathrm{~mm}$ Width of flange, $b=1000 \mathrm{~mm}$ Width of web, $b_{w}=300 \mathrm{~mm}$ Effective flange width,
$b_{r}=\frac{L_{0}}{\frac{L_{0}}{b}+4}+b_{w}$
$=\frac{9000}{9+4}+300$
$=992.31 \mathrm{~mm}<(1000 \mathrm{~mm})$
Modular ratio (For M20 concrete) m $=13.33$
Area of tension reinforcement,
$A_{z}=6 \times \pi \times \frac{25^{2}}{4}=2945.25 \mathrm{~mm}^{2}$
Effective depth, d $=620 \mathrm{~mm}$
Depth of flange, $b_{w}=300 \mathrm{~mm}$
Df $=150 \mathrm{~mm}$
Let the depth of neutral axis is $\mathrm{x}_{\mathrm{a}}$.
The neutral axis is located in the web
$\therefore \mathrm{X}_{\mathrm{a}}>\mathrm{D}_{\mathrm{f}}$
Equating area-moment about the
Neutral axis for compression and tension side.
$\Rightarrow 992.3 \times 150 \times\left(x_{2}-75\right)+\frac{\left(x_{2}-150\right)^{2}}{2} \times 300$
$=13.33 \times 2945.25 \times\left(620-x_{2}\right)$
$\Rightarrow 150\left(x_{2}\right)^{2}+143105.1825 x_{2}-32129688.15$
$x_{a}=187.62 \mathrm{~mm}$
So, the depth of neutral axis
$=187.62 \mathrm{~mm}$
42. A cylindrical specimen of 30 mm dia and 80 mm length is prepared from dry soil. If specimen has moisture content $10 \%$ and air void $15 \%$. Find
weight of dry soil (in grams) required if specific gravity $=2.7$.
A. 113
B. 102
C. 110
D. 65

Ans. B
Sol. vol. of soil sample $=\frac{\Pi}{4} \times 0^{2} \times 4$
$=\frac{\Pi}{4} \times 3^{2} \times 8=56.54 C C$
Water content
$(w)=\frac{w w}{w s}=10 \%=0.1$
$\eta a=0.15=\frac{\text { Vair }}{\text { vol. of soil }}=\frac{V a}{V}$
$V a=0.15 \times 56.54=8.48 c c$
Now
$w=\frac{w W}{w s}=\frac{V w \times y w}{v s \times y s}=\frac{v w}{G V s}$
$\frac{v w}{v s}=g w$
$\Rightarrow \frac{v W}{v s}=2.7 \times 0.1=0.27$

$$
v w+v s=56.54-8.48
$$

$$
v w+v s=48.06 c c
$$

$0.27 v s+v s=48.06 c c$

$$
1.27 V s=48.06 C C
$$

$$
v s=37.84 c c
$$

$\therefore V s=\frac{w s}{G T w}$
$\Rightarrow W s=V s \times G T W=37.84 \times 2.7 \times 1$

$$
=102.17 \mathrm{~g}
$$

43. CU triaxial tests conducted on specimens of a saturated clay soil gave the following results

| Test | Cell Pressure $\left(\mathrm{kN} / \mathrm{m}^{2}\right)$ | Deviator Stress $\left(\mathrm{kN} / \mathrm{m}^{2}\right)$ | Pore Water Pressure $\left(\mathrm{kN} / \mathrm{m}^{2}\right)$ |
| :---: | :---: | :---: | :---: |
| 1. | 140 | 120 | 70 |
| 2. | 280 | 200 | 140 |

The effective stress parameters $c^{\prime}$ and $\varphi^{\prime}$ will be
A. $15.95,12.84^{\circ}$
B. $13.67,21.32^{\circ}$
C. $16.83,10.24^{\circ}$
D. $11.32,23.54^{\circ}$

Ans. B

Sol. From Mohr's circle
$\sigma_{1}{ }_{1}=\sigma_{3} \tan ^{2}\left(45+\frac{\Phi^{\prime}}{2}\right)+2 c^{\prime} \tan (45$
$\left.+\frac{\Phi^{\prime}}{2}\right)$
For test (1)
$\sigma_{3}=140 \mathrm{kPa}, \sigma_{\mathrm{d}}=120 \mathrm{kPa}, \mathrm{u}=70$
kPa
Thus, $\sigma^{\prime}{ }_{3}=\sigma_{3}-\mathrm{u}=140-70=70$
kPa
$\sigma_{1}{ }_{1}=\sigma_{1}-u=(140+120)-70=190$
kPa
For test (2)
$\sigma_{3}=280 \mathrm{kPa}, \sigma_{\mathrm{d}}=200 \mathrm{kPa}, \mathrm{u}=140$
kPa
Thus, $\sigma^{\prime}{ }_{3}=\sigma_{3}-\mathrm{u}=280-140=$ 140 kPa
$\sigma_{1}=\sigma_{1}-u=(280+200)-140=$ 340 kPa
From equation (i)
$190=70 \tan ^{2}\left(45+\frac{\Phi^{\prime}}{2}\right)+2 c^{\prime} \tan$
$\left(45+\frac{\Phi^{\prime}}{2}\right)$
$340=140 \tan ^{2}\left(45+\frac{\Phi^{\prime}}{2}\right)+2 c^{\prime} \tan$
$\left(45+\frac{\Phi^{\prime}}{2}\right)$
On solving the above equations, we get
$\tan ^{2}\left(45+\frac{\Phi^{\prime}}{2}\right)=2.143$
Or, $\varphi^{\prime}=21.32^{\circ}$ and $c^{\prime}=13.67$
44. A square footing of having size 2.5 m rests on a dense sand which has angle of internal friction $\varnothing=40^{\circ}$. The depth of the foundation being 1.5 m . The unit weight of the sand above the water table is $17 \mathrm{kN} / \mathrm{m}^{3}$ and the saturated unit weight of soil is 22 $\mathrm{kN} / \mathrm{m}^{3}$. Values of bearing capacity parameters for $\varnothing=40^{\circ}$ are $N_{q}=81.3$, $N_{\mathrm{Y}}=100.4$
If the water table rests at the footing level, the ultimate bearing capacity of the soil is
A. $3162 \mathrm{kN} / \mathrm{m}^{2}$
B. $3297 \mathrm{kN} / \mathrm{m}^{2}$
C. $2998 \mathrm{kN} / \mathrm{m}^{2}$
D. $3335 \mathrm{kN} / \mathrm{m}^{2}$

Sol. When the water table is at the footing level, the soil below the footing i.e. in zone 2 will be submerged while the soil above the footing i.e. in zone 1 will be unaffected.
So $\gamma_{q}=\gamma_{t}=17 \mathrm{kN} / \mathrm{m}^{3}$ and $\gamma_{\mathrm{s}}=\gamma^{\prime}=$
$Y_{\text {sat }}-9.81=12.19 \mathrm{kN} / \mathrm{m}^{3}$
Now $q_{u}=1.3 \mathrm{cN}_{\mathrm{c}}+\mathrm{Y}_{\mathrm{q}} \mathrm{D}_{\mathrm{f}} \mathrm{N}_{\mathrm{q}}+$
$0.4 \gamma_{s} B N_{Y}$
$=0+17 \times 1.5 \times 81.3+$
$0.4 \times 12.19 \times 2.5 \times 100.4$
$=2073.15+1223.876=3297.026$
kN/m ${ }^{2}$
45. Find the load capacity of a group of 9 piles, arranged into a $3 \times 3$ square pattern, if the center to center distance of each pile is 900 mm and the adhesion factor is 0.9 . The diameter of all the nine piles is 300 m and the length of each pile is 8 m . The soil type is medium stiff clay having an undrained compressive strength of $100 \mathrm{kN} / \mathrm{m}^{2}$.
A. 1336 KN
B. 1450 KN
C. 1300 kn
D. 1475 KN

Ans. A
Sol. Given,
Diameter of the pile $=\mathrm{D}=300 \mathrm{~mm}=$ 0.3 m

Length of the pile $=8 \mathrm{~m}$
Adhesion factor $\mathrm{a}=0.9$
$\mathrm{C}=\frac{\frac{q u}{2}}{2}=50 \mathrm{KN} / \mathrm{m}^{2}$
$B=2 \times 0.9+0.3=2.1 \mathrm{~m}$
Ultimate load of group pile acting together,
$\mathrm{Q}_{\mathrm{g} 1(\mathrm{U})}=\mathrm{q}_{\mathrm{p}} \mathrm{A}_{\mathrm{g}}+\mathrm{c}(4 \mathrm{BL})$
$=c N_{c} B^{2}+c(4 B L)$
$=(50 \times 9) \times 2.1^{2}+50 \times 4 \times 2.1 \times 8=$ 5344.5KN

Ultimate load of pile with each pile acting individually
$\mathrm{Q}_{\mathrm{g} 2(\mathrm{u})}=\mathrm{NQ}_{\mathrm{u}}$
$=9\left(c N_{c} A_{p}+a c(n D L)\right)$
$=9\left(50 \times 9 \times\left(n \times 0.3^{2} / 4\right)+\right.$
$0.9 \times 50 \times \pi \times 0.3 \times 8$ )
$=3339.9 \mathrm{KN}$

Ans. B

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As the ultimate load for individual pile failure case is less than the pile in group load, safe load is given by $\mathrm{Q}_{\mathrm{a}}=3339.9 / \mathrm{FOS}$
=3339.9/2.5
$=1336 \mathrm{kN}$
46. A rectangular channel of width 4 m is carrying a discharge of $36 \mathrm{~m}^{3} / \mathrm{sec}$. If the depth of flow of one end of hydraulic jump is 1.2 m then what is the power dissipated in the jump?
A. 255.77 KW
B. 298.27 KW
C. 176.58 KW
D. 198.27 KW

Ans. C
Sol. Froude number for rectangular channel
$F_{1}^{2}=\frac{q^{2}}{g Y_{1}^{3}}$, here $\mathrm{q}=\mathrm{Q} / \mathrm{B}=36 / 4=9$
$F_{1}^{2}=\frac{9^{2}}{9.81 \times 1.2^{3}}=4.78$
Depth of flow of another end of hydraulic jump
$\frac{Y_{2}}{Y_{1}}=\frac{1}{2}\left[\left(\sqrt{1+8 F_{1}^{2}}\right)-1\right]$
$\frac{Y_{2}}{1.2}=\frac{1}{2}[(\sqrt{1+8 \times 4.78})-1]$
$Y_{2}=3.16 \mathrm{~m}$
Energy dissipation
$E_{L}=\frac{\left(Y_{2}-Y_{1}\right)^{3}}{4 Y_{1} Y_{2}}=\frac{(3.16-1.2)^{3}}{4 \times 1.2 \times 3.16}=0.5 \mathrm{~m}$
Power dissipated in jump
$P_{L}=\gamma Q E_{L}=9.81 \times 36 \times 0.5=$ 176.58 KW
47. Find the settling velocity of a discrete particle in water under conditions when Reynold's number is 112 . The diameter and specific gravity of the particle is $5 \times 10^{-2} \mathrm{~cm}$ and 2.65, respectively. Water temperature is $20^{\circ}$ Celsius.
A. $.22 \mathrm{~m} / \mathrm{sec}$
B. $.11 \mathrm{~m} / \mathrm{sec}$
C. $.45 \mathrm{~m} / \mathrm{sec}$
D. $.24 \mathrm{~m} / \mathrm{sec}$

Ans. C
Sol. When Reynold's number is greater than 1 and less than 100000 the flow is in transition.
$V_{s}=\sqrt{\frac{4(G-1) g d}{3 \times C_{d}}}$
$C_{d}=\frac{24}{R_{e}}+\frac{3}{\sqrt{R_{e}}}+.34=$
Hence, $V_{s}=.11 \mathrm{~m} / \mathrm{s} \ldots . \mathrm{i}$
Or
$V_{s}=\frac{418(G-1) d}{1} \times \frac{3 T+70}{100}(\mathrm{~d}$ is in mm
and T in Degree Celsius)
$V_{s}=.448 \mathrm{~m} / \mathrm{s} \ldots \mathrm{ii}$
Maximum of I and ii, there for Vs is $.448 \mathrm{~m} / \mathrm{s}$
48. If a $8^{0}$ curve track diverges from a main curve of $5^{0}$ in an opposite direction in the layout of a B.G. yard calculate the super-elevation if maxi mum speed permitted on the main line is 50 kmph .
A. 1.69 cm
B. 1.99 cm
C. 1.15 cm
D. 2.31 cm

Ans. B
Sol. Equilibrium cant required for 45
kmph speed, from the equation
$\mathrm{e}=\mathrm{GV}^{2} / 1.27 \mathrm{R}$
Where,
$\mathrm{G}=1.676 \mathrm{~m}$ for B.G
$\mathrm{V}=40 \mathrm{kmph}$
$\mathrm{R}=1720 / 5$
Hence, $\mathrm{e}=(1.676 \times 50 \times 50 / 1.27) \times$
5/1720
$=9.59 \mathrm{~cm}$
For broad gauge the cant deficiency permitted for main line is 7.6 cm
So the cant for main track $=9.59$ -
$7.6=1.99 \mathrm{~cm}$
Therefore the cant to be provided for branch track $=-1.99 \mathrm{~cm}$
It is a negative cant of 1.99 cm
49. A valley curve is as shown below


If the stopping sight distance is 100 m , the length of the valley curve for the headlight distance criteria is
$\qquad$ m. [rounded to one decimal place]
Sol. Assume length of curve $\mathrm{L}_{\mathrm{v}}>$ SSD
$\mathrm{S}=100 \mathrm{~m} ; \mathrm{N}=\left|\mathrm{N}_{2}-\mathrm{N}_{1}\right|=\frac{1}{30}-\left(\frac{-1}{25}\right)=\frac{11}{150}$
$\mathrm{L}=\frac{\mathrm{NS}^{2}}{2 \mathrm{~h}_{1}+2 \operatorname{stan} \alpha}=\frac{\frac{11}{150} \times 100^{2}}{2 \times 0.7+2 \times 100 \times \tan \left(1.5^{9}\right)}=110.488 \mathrm{~m}$
$\mathrm{L}=110.5 \mathrm{~m}>(\mathrm{SSD}=100 \mathrm{~m})$
Hence $L=110.5 \mathrm{~m}$
50. Determine the theoretical oxygen demand in $\mathrm{mg} / \mathrm{l}$ of fructose solution which is present in quantity $600 \mathrm{mg} / \mathrm{l}$ of water.
A. 192
B. 180
C. 640
D. 384

Ans. C
Sol. Mol Wt. of Fructose $\left(\mathrm{C}_{6} \mathrm{H}_{12} \mathrm{O}_{6}\right)=180$ gms
Reaction
$\mathrm{C}_{6} \mathrm{H}_{12} \mathrm{O}_{6}+6 \mathrm{O}_{2} \tilde{\mathrm{~A}} 6 \mathrm{CO}_{2}+6 \mathrm{H}_{2} \mathrm{O}$
1 mole of fructose requires 6 moles
of $\mathrm{O}_{2}=192 \mathrm{gms}$.
Oxygen demand,
$\frac{600}{180} \times 192=640 \mathrm{mg} / \mathrm{l}$
51. Match list I with list II and select the correct answer using the codes given below:

| LIST I | LIST II |
| :--- | :--- |
| A. Correction for sag | 1. Tacheometer |
| B. Least count $30^{\prime}$ | 2. Aerial photograph |
| C. overlap | 3. base line |
| D. additive constant | 4. prismatic compass |

A. A-4, B-3, C-2, D-1
B. $A-3, B-4, C-2, D-1$
C. A-1, B-2, C-3, D-4
D. A-3, B-4, C-1, D-2

Ans. B
Sol. In a triangulation, baseline is measured very precisely.

- The sag correction is used for baseline measurement.
- Temperature correction is applied by measuring the temperature correction at least at three places.
- Finally, slope correction is also needed on steep slopes.
- The overlap of the photo are maintained to ensure complete coverage of the area.
- The overlap in the direction of light is known as longitudinal or forward overlap (about 60\%).
The overlap between adjacent flights is known as side overlap (about $30 \%$ ). It is used to :
i) Orient point so as to form a continuous flight slip.
ii) View the photograph by stereoscope.

52. Determine the distance between instrument station $X$ and the staff station $Y$ when staff readings are $1.231 \mathrm{~m}, 0.823 \mathrm{~m} \& 0.350 \mathrm{~m}$. RL. of line of collimation is 200.150 m and vertical angle is (- $4^{\circ} 45^{\prime}$ ). Also determine the RL. Of ' $Y$ '. Take $\mathrm{K}=100$ and $\mathrm{C}=0.8$.
A. $88.3 \mathrm{~m}, 206.657 \mathrm{~m}$
B. $88.3 \mathrm{~m}, 191.997 \mathrm{~m}$
C. $87.4 \mathrm{~m}, 191.997 \mathrm{~m}$
D. $87.4 \mathrm{~m}, 206.657 \mathrm{~m}$

Ans. B
Sol. Horizontal distance (For angle of depression)
$D=K S \cos ^{2} \theta+C \cos \theta$
Here, $S=(1.231-0.350)=0.881 \mathrm{~m}$
D $=\left[100 \times 0.881 \times \cos ^{2}\left(4^{\circ} 45^{\prime}\right)\right]+$
[0.8 $\times \cos \left(4^{\circ} 45^{\prime}\right)$ ]
$=88.3 \mathrm{~m}$
Vertical distance, $\mathrm{V}=[(\mathrm{KS}$
$\sin 2 \theta) / 2]+C \sin \theta$
$=[(100 \times 0.881 \times 0.165) / 2]+(0.8$
$\times 0.083$ )
$=7.33 \mathrm{~m}$
RI. of $Y=$ RL. of line of collimation -
V - Staff reading
$=$ 200.150-7.33-0.823
$=191.997 \mathrm{~m}$
53. Calculate strength of a 20 mm diameter bolt of grade 4.6 for the following:
The main plate to be joined are 12
mm thick with lap joint (Assume end distance $e=33 \mathrm{~mm}$ and pitch $p=50$ $\mathrm{mm} \& ~ t h r e a d$ intercept shear plane)

A. 45.26 kN
B. 96 kN
C. 101 kN
D. 83 kN

Ans. A
Sol. Shank diameter of bolt $\mathrm{d}=20 \mathrm{~mm}$ Diameter of bolt hole $d_{0}=20+2=22$ mm
For grade 4.6 bolt, $f_{u b}=400 \mathrm{MPa}$ $\mathrm{t}=12 \mathrm{~mm}, \mathrm{e}=33 \mathrm{~mm} \& \mathrm{p}=50 \mathrm{~mm}$ When thread intercepts the shear plane
$\mathrm{n}_{\mathrm{n}}=1 \& \mathrm{n}_{\mathrm{s}}=0$ for lap connection in case of one bolt.
Design strength of one bolt,
$\mathrm{V}_{\mathrm{db}}=$ Lesser of $\mathrm{V}_{\mathrm{dsb}}$ or $\mathrm{V}_{\mathrm{dpb}}$
Design shear strength of one bolt,
$\mathrm{V}_{\mathrm{dsb}}=$
$\frac{f_{u b}}{\sqrt{3} Y_{m b}}\left(n_{n} A_{n b}+n_{s} A_{s b}\right)=\frac{400}{\sqrt{3} * 1.25}\left(1 * 1 * 0.78 * \frac{\pi}{4} * 20^{2}+0\right)$
$=45.26 \mathrm{kN}$
Design bearing strength of one bolt,
$\mathrm{V}_{\mathrm{dpb}}=2.5 \mathrm{k}$.b.d.t.fub $/ Y_{m b}$
$k_{b}$ is a bearing factor is lesser
of $\frac{e}{3 d_{o}}=\frac{33}{3 * 22}=0.50$
$\frac{p}{3 d_{o}}-0.25=\frac{50}{3 * 22}-0.25=0.507$
$\frac{\text { Fub }}{\mathrm{fu}}=\frac{400}{410}=0.97 \sim 1.0$
$k_{b}=0.50$
$\mathrm{V}_{\mathrm{dpb}}=2.5 \mathrm{k} . \mathrm{b} . \mathrm{d} . \mathrm{t} . \mathrm{f}_{\mathrm{ub}} / Y_{m b}$
$=2.5 * 0.5 * 20 * 12 * 400 / 1.25=96 \mathrm{kN}$
Design strength of one bolt
, $\mathrm{V}_{\mathrm{db}}=$ Lesser of $\mathrm{V}_{\mathrm{dsb}}$ or $\mathrm{V}_{\mathrm{dpb}}=45.26 \mathrm{kN}$
54. A bracket has been attached to flange of a column as shown in the figure. The maximum resultant force in the bolt is

A. $P / 4$
B. $P / 2$
C. $3 P / 4$
D. $P$

Ans. C
Sol. $\mathrm{F}_{\mathrm{a}}=\frac{P}{4}-\frac{P}{4}=0$
$\mathrm{F}_{\mathrm{m}}=\frac{\text { Per }}{\Sigma r^{2}}$
$r=\sqrt{80^{2}+60^{\wedge} 2}=100$
$\mathrm{F}_{\mathrm{m}}=\frac{\text { Per }}{\sum r^{2}}==\frac{P * 300 * 100}{4 * 100 * 100}=3 \mathrm{P} / 4$
$\mathrm{F}_{\mathrm{R} \max }=\sqrt{F a^{2}+\mathrm{Fm}^{2}+2 F a F m \cos \theta}$
$F_{R}=F_{m}=3 P / 4$
55. A projectile is fired horizontally at $8 \mathrm{~m} / \mathrm{s}$ from a point of $h$ meter height above and 20 m away from the target. What is the value of $h$ required so that the projectile hits the target?
A. 30.25
B. 31.75
C. 30.75
D. 31.25

Ans.
Sol. At initial position, $\mathrm{u}_{\mathrm{x}}=8 \mathrm{~m} / \mathrm{s}$

$S_{x}=20 \mathrm{~m}$
So, $\mathrm{S}_{\mathrm{x}}=\mathrm{u}_{\mathrm{x}} \mathrm{t}$ [t=time taken to hit the target]
Or, $\mathrm{t}=\frac{20}{8} \mathrm{sec}=2.5 \mathrm{sec}$
There is no initial velocity in $Y$ direction
So, $\mathrm{S}_{\mathrm{y}}=\mathrm{u}_{\mathrm{y}} \mathrm{t}+\frac{1}{2} \mathrm{X} \mathrm{g} \mathrm{X} \mathrm{t}{ }^{2}\left[\mathrm{u}_{\mathrm{y}}=0\right]$
So, $S_{y}=h=\frac{1}{2} \times 10 \times 2.5^{2}=31.25 \mathrm{~m}$
56. The given
integral
$\int_{0}^{\frac{\pi}{k}} \int_{x}^{\frac{\pi}{k}} \frac{\sin y}{y} d y d x$ evaluates to $\frac{1}{2}$
for some $k \geq 1$. Then the value of $k$ is:
Sol. Changing the order of integration we get:

$$
\begin{aligned}
& \int_{0}^{\frac{\pi}{k}} \int_{0}^{y} \frac{\sin y}{y} d x d y \\
& =\int_{0}^{\frac{\pi}{k}} \frac{\sin y}{y} X[x]_{0}^{y} d y \\
& =\int_{0}^{\frac{\pi}{k}} \sin y d y \\
& =[-\cos y]_{0}^{\frac{\pi}{k}}=-\cos \frac{\pi}{k}+1 \\
& -\cos \frac{\pi}{k}+1=\frac{1}{2} \\
& \text { giving } \frac{\pi}{k}=\cos ^{-1} 0.5
\end{aligned}
$$

$\frac{\pi}{k}=\frac{\pi}{3}$
thus, $k=3$
57. The line integral of the vector field $\mathrm{F}=$ $5 x z i+\left(3 x^{2}+2 y\right) j+x^{2} z k$ along a path from $(0,0,0)$ to $(1,1,1)$ parametrized by $\left(t, t^{2}, t\right)$ is $\qquad$ .
Sol. $\quad F=5 x z \bar{i}+\left(3 x^{2}+2 y\right) \bar{j}+x^{2} z=\bar{k}$
$x=t ; y=t^{2} ; z=t$
$\Rightarrow d x=d t ; d y=2 t d t \& d z=d t$
$\therefore$ The line integral of the vector field is
$\int_{c} \bar{F} \cdot \overline{d r}=\int 5 x-d x+\left(3 x^{2}+2 y\right) d y+\left(x^{2} z\right) d t$
$\int_{0}^{1}\left(5 t^{2} d t+10 t^{3} d t+t^{3} d t\right)$
$\int_{0}^{1}\left(5 t^{2} d t+11 t^{3} d t\right)$
$=5\left[t^{3} / 3\right]_{0}^{1}+11\left[t^{4} / 4\right]_{0}^{1}$
$=5(1 / 3)+11(1 / 4)$
$=5 / 3+11 / 4=\frac{20+33}{12}=\frac{53}{12}=4.4167$
58. $X$ and $Y$ are two continuous random variable with joint distribution:
$f(x, y)= \begin{cases}c x+1, & y \geq 0 \& x+y<1 \\ 0, & \text { otherwise }\end{cases}$
The value of the constant c is:
Sol. For a joint probability distribution, we must have:

$$
\begin{aligned}
& \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} f(x, y) d x d y=1 \\
& \int_{0}^{1} \int_{0}^{1-x}(c x+1) d x d y=1 \\
& \int_{0}^{1}(c x+1)(1-x) d x=1 \\
& \text { giving } \frac{1}{2}+\frac{c}{6}=1 \\
& \text { Thus: } c=3
\end{aligned}
$$

59. From the given data, the maximum resources load in any week will be weeks.

| Activity | Start week | End week | Resources needed per week |
| :---: | :---: | :---: | :---: |
| A | $1^{\text {st }}$ | $7^{\text {th }}$ | 12 |
| B | $1_{\text {st }}$ | $10^{\text {th }}$ | 16 |
| C | $8^{\text {st }}$ | $13^{\text {th }}$ | 13 |
| D | $9^{\text {st }}$ | $15^{\text {th }}$ | 10 |

Sol. For $1^{\text {st }}$ to $7^{\text {th }}$ week both activity A \& $B$ will progress, hence, resource needed $=12+16=28$
For $8^{\text {th }}$ week both activity B \& C will progress, hence, resource needed $=16+13=29$
For $9^{\text {th }}$ to $13^{\text {th }}$ week activities B, C \& $D$ will be in progress, hence,
resource needed $=16+13+10=39$

For $11^{\text {th }}$ to $13^{\text {th }}$ week both activity C \& $D$ will be in progress, hence, resource needed $=13+10=26$ Therefore, Maximum resources needed $=39$
60. A rigid wheel 1.25 m in diameter is to be provided with a thin steel tyre. If the stress in the steel type is not to exceed $140 \mathrm{MN} / \mathrm{m}^{2}$, the minimum temperature to which the type is to be raised so that it can be fitted over the wheel is $\qquad$ ${ }^{\circ} \mathrm{C}$. Take $\mathrm{E}=200$ $\mathrm{GN} / \mathrm{m}^{2} ; \propto=12 \times 10^{-6} /{ }^{\circ} \mathrm{C}$.
Sol. Let $\mathrm{D}=$ Diameter of rigid wheel
$D=$ least diameter of steel tyre
Strain in tyre $=\frac{D-d}{d}$
Strain in tyre $=\left(\frac{D-d}{d}\right) E=\sigma$
$\therefore \frac{\mathrm{D}}{\mathrm{d}}-1=\frac{\sigma}{\mathrm{E}}$
$\frac{D}{d}-1=\frac{140 \times 10^{6}}{200 \times 10^{9}}=0.0007$
$\frac{\mathrm{D}}{\mathrm{d}}=1.0007$
Let the steel be subjected to a temperature rise of $\mathrm{T}^{\circ} \mathrm{C}$.
$\pi D=\pi d+\pi d \cdot \propto T$
$\frac{\pi D}{\pi d}=1+\infty T$
$\frac{D}{d}=1+\infty T$
$1.0007=1+\propto T$
$\mathrm{T}=\frac{0.0007}{12 \times 10^{-6}}=58.33^{\circ} \mathrm{C}$
61. A simply supported beam of span 8 meter having a rectangular cross section of $500 \mathrm{~mm} * 250 \mathrm{~mm}$. The beam is prestressed with force 1500 KN at 50 mm eccentricity below the neutral axis .What is the position in mm below the neutral axis at centre of span where tensile stress is zero? Considering the self weight of material of beam is $26 \mathrm{KN} / \mathrm{m}$.

Sol. Moment due to self weight at centre of span $=\mathrm{wl}$ ^2/8 $=26^{*} 8^{\wedge} 2 / 8=$ 208 KNm
The tensile stress $\mathrm{f}_{\mathrm{t}}=\mathrm{P} / \mathrm{A}-\mathrm{M}^{*} \mathrm{y} / \mathrm{I}+$ P*e*y/I
$=(1500 * 10 \wedge 3) /(500 * 250)-(208 *$
10^6 *y/(250*500^3/12) +
(1500*10^3 * 50*y /
(250*500^3/12)
= $12-0.0798 y+0.0288 y$
Equate the equation to zero ; $y=235.29 \mathrm{~mm}$
62. A standard penetration test was conducted in a soil having saturated density $18 \mathrm{kN} / \mathrm{m}^{3}$, ground water table was found at a depth of 4.5 m . If the N value was determined as 28 at the depth of 5 m , then the corrected N value of the soil is
Sol. $\bar{\sigma}=4.5 \times \gamma_{\text {sat }}+0.5 \times \gamma^{\prime}$
$=4.5 \times 18+0.5 \times(18-9.81)$
$=85.095$
Hence overburden correction is required.
$\mathrm{N}_{1}=\mathrm{N}_{0} \times \frac{350}{\bar{\sigma}+70}$
350
$=28 \times \frac{350}{85.095+70}$
$=63.187$
Applying dilatancy correction,
$N_{2}=15+\frac{1}{2}\left(N_{1}-15\right)$
$N_{2}=15+\frac{1}{2}(63.22-15)$
$N_{2}=39.11$
63. Rainfall over a basin in three consecutive hours are $3.5 \mathrm{~cm}, 4 \mathrm{~cm}$ and 3 cm respectively. The infiltration loss can be estimated using the following Horton equation
$\mathrm{f}=0.9+3.8 \mathrm{e}^{-2 \mathrm{t}}$
where f is infiltration in $\mathrm{cm} / \mathrm{hr}$ and $\mathrm{t}=$ time in hour from start of rainfall.
Assuming negligible surface retention and evaporation losses, the surface run off from the basin is $\qquad$ cm
[Rounded to two decimal places]

Sol. Given,
Rainfall intensity for $1^{\text {st }}$ hour $\frac{3.5}{1}=$

## $3.5 \mathrm{~cm} /$ hour

Rainfall intensity for $2^{\text {nd }}$ hour $=\frac{4}{1}=$ $4 \mathrm{~cm} /$ hour
Rainfall intensity for $3^{\text {rd }}$ hour $=\frac{3}{1}=3$ cm/ hour
Calculate time when Horton curve cuts hyetograph.
Assuming that curve cuts the
hyetograph before $t=1$ hour
$0.9+3.8 e^{-2 t}=3.5$
$t=0.1897$ hours
Hence till $t=0.1018$ hours, there
will be no run off
Total infiltration $F(t)=3.5 \times 0.1897$
$+\int_{0.1897}^{3}\left(0.9+3.8 \mathrm{e}^{-2 \mathrm{t}}\right) \mathrm{dt}$
$\mathrm{F}(\mathrm{t})=0.664+\int_{0.1897}^{3} 0.9 \mathrm{dt}+\int_{0.1897}^{3} 3.8 \mathrm{e}^{-2 \mathrm{t}} \mathrm{dt}$
$F(t)=0.664+0.9(3-0.1897)+3.8 \int_{0.1897}^{3} e^{-2 t} \cdot d t$
$F(t)=0.664+2.53+3.8 \times 0.3409$
$F(t)=4.489 \mathrm{~cm}$
Hence Runoff $=$ Total precipitation -
Infiltration
$=(3.5+4+3)-4.489$
Runoff $=6.01 \mathrm{~cm}$
64. A tank 1.5 m high stands of a trolley and is full of water. It has an orifice of diameter 0.1 m at 0.3 from the bottom of the tank. If the orifice is suddenly opened, the propelling force on the trolley is $\qquad$ N. [Take coefficient of discharge of the orifice as 0.60 ]

Sol. Discharge from the orifice=

$$
\begin{aligned}
& C_{d} a \sqrt{2 g H} \\
& =0.60 \times \frac{\pi}{4}(0.1)^{2} \times \sqrt{2 \times 9.81 \times 1.2} \\
& =0.023 \mathrm{~m}^{3} / \mathrm{s}
\end{aligned}
$$

Velocity of the jet issuing from the orifice
$=\frac{\mathrm{Q}}{\mathrm{a}}=\frac{0.023 \times 4}{\pi \times(0.1)^{2}}=2.93 \mathrm{~m} / \mathrm{s}$
Propelling force, $\mathrm{F}={ }^{\rho} \mathrm{QV}$
$=1000 \times 0.023 \times 2.93$
$=67.39 \mathrm{~N}$
65. A completely mix process is designed with the following data:
Flow of sewage $\left(\mathrm{Q}_{0}\right)=10000 \mathrm{~m}^{3} /$ day, BOD after PST=150mg/I, Effluent BOD $=5 \mathrm{mg} / \mathrm{l}$
$Y=.5 \mathrm{Kg} / \mathrm{kg}$
$\mathrm{K}_{\mathrm{d}}=.05$ per day
MLSS $=3000 \mathrm{mg} / \mathrm{l}$.
Under flow concentration ( $\mathrm{X}_{\mathrm{u}}$ ) = $10000 \mathrm{mg} / \mathrm{l}$ for secondary sedimentation tank, calculate the sludge wasted (in cumec per day). (Consider sludge age $=8$ days)
We know, $Q_{w} X_{u}=\frac{V X}{\theta}, \ldots . \mathrm{i}$
$V X=\frac{Q_{0}\left(S_{0}-S\right) \theta \times Y}{1+.05 \theta} \ldots . . . \mathrm{ii}$
$V \times 3000=\frac{10000 \times(150-5) \times 8 \times .5}{1+.05 \times 8}$
$V=1380.95 \mathrm{~m}^{3}$
by putting value of $V$ ineq. $i, Q_{w} X_{u}=\frac{1380.95 \times 10^{3} \times 3000 \times 10^{-6}}{8}$

$$
=517.88 \frac{\mathrm{~kg}}{\mathrm{day}}
$$

$Q_{W}=\frac{517.88}{10^{4} \times 10^{-6}} \times 10^{-3}=51.786 \mathrm{~m}^{3} /$ day

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